MAGNITUDE AND FREQUENCY OF LOW STREAMFLOWS IN NEW HAMPSHIRE

DTIC QUALITY INSPECTAD &



DEPARTMENT OF THE ARMY NEW ENGLAND DIVISION, CORPS OF ENGINEERS WALTHAM, MASS.

DECEMBER 1980

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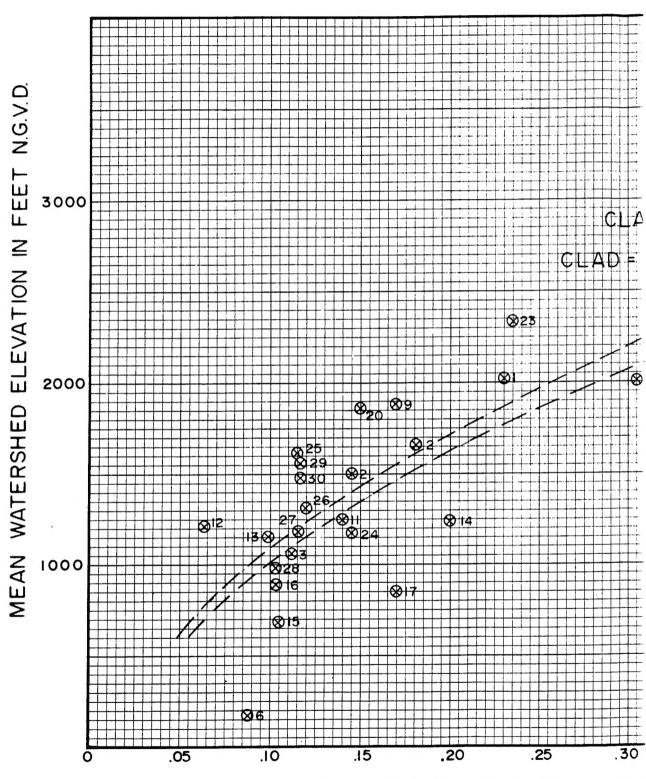
FOREWORD

This study was conducted for the State of New Hampshire under Section 22 Authority. It was performed by Anderson Nichols Co., Inc. under contract to the Hydrologic Engineering Section of the Water Control Branch. The study was designed to be exploratory in nature and the findings are reported for use and comment by field personnel. Responses to the study report will be used in determining the need and direction of any further studies.

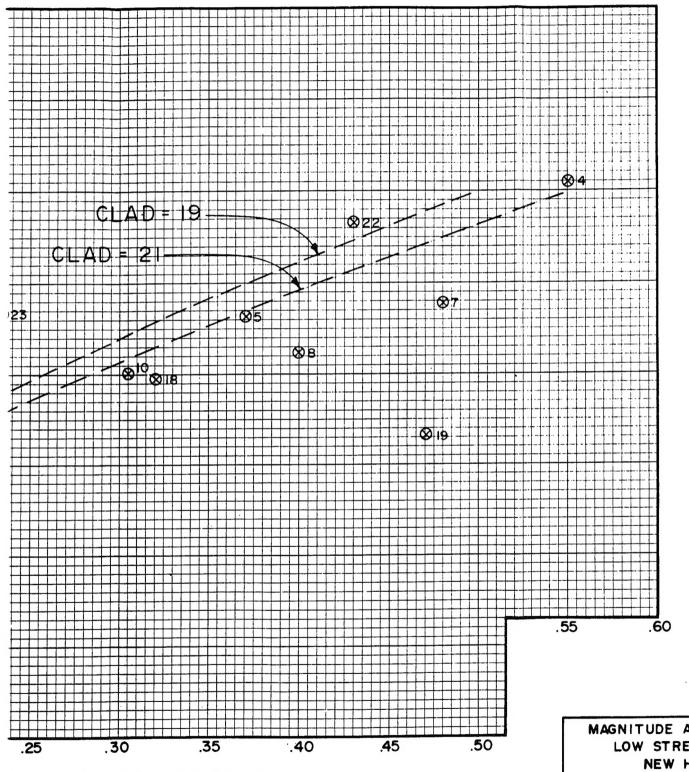
The main report discusses regression analyses made of regional low flow data. Appendix "A" presents the regional low flow data in graphical form and Appendix "B" contains summary computer printouts of the "WATSTOR" low flow data.

Supplemental comments and recommendations are as follows:

- 1. The developed regression equations indicate zero or negative low flows for watershed at or below elevation 600 feet NGVD, and the author suggested (page 59) that, in such cases, a minimum flow value of 0.0001 csm be assumed. It is now concluded that there is no basis for selecting this value and it is probably excessively low. Since much of the basic data used in the analyses was "bunched" about the 1,000 foot elevation (see attachments 1 and 2) there is little rationale for extrapolation below this elevation. It is therefore recommended that, for watersheds less than 1,000 feet in elevation, a minimum "E" of 1,000 feet be adopted for use in the developed equations.
- 2. It is emphasized that the watershed elevation "E", used in the analyses is not the elevation of the stream at the point of interest but is the mean elevation of the contributing watershed (see page 39).
- 3. The term, "usable storage", as used in this report, refers to the approximate regulated storage capacity of a reservoir. Such maximum storage capacity should not be interpreted, in all cases, as necessarily usable, as the term might apply.



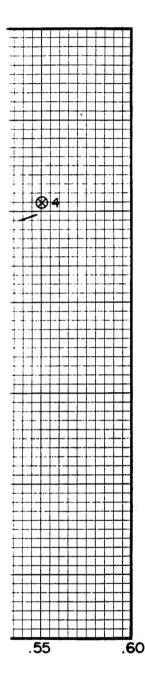
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LIST OF PLOTTED DATA ON ATTACHMENT I

(See pages 1 and 2, Appendix A for Station Index)

Plot	USGS Station	Mean Watershed Elevation (ft. NGVD)	2 year Frequency (50%) 7 Day Low Flow (cfs/sq. mi.)
1	01052500	2030	.21
2	01054300	1654	.16
2	01057000	1053	.09
4	01064300	3050	.53
5	01064500	2320	.35
6	01073000	170	.06
6 7	01074500	2390	.46
	01075000	2120	.38
8 9	01076000	1890	.15
10	01076500	2010	.28
ii	01078000	1250	.12
12	01084500	1214	.04
13	01086500	1150	.08
14	01087000	1240	.18
15	01089000	680	.08
16	01091000	889	.08
17	01094000	850	.15
18	01130000	1970	.30
19	01133000	1666	.45
20	01134500	1865	.13
21	01135000	1500	.12
22	01137500	2840	.41
23	01138000	2340	.21
24	01142500	1165	.12
25	01145000	1610	.09
26	01153500	1300	.10
27	01154000	1171	.09
28	01155000	960	.08
29	01156000	1561	.09
30	01160000	1480	.09

ACKNOWLEDGMENT

This study and report was prepared by Anderson-Nichols & Co., Inc. Concord, New Hampshire under contract to the New England Division, Corps of Engineers, Waltham, Massachusetts.

CONTENTS

- I. Introduction
 Purpose of Study
 Scope of Study
- II. Literature Review
 Introduction
 Research Methodologies
 Flow Duration Curves
 Watershed Characteristics and Low Flows
 Conclusions
- III. Regional Analysis of Low Flow Discharge Rates
 Description of Physical Environment
 Geomorphology
 Climate
 General Processes that Control Watershed Low Flows
 Low Flow Modeling
 Selection of Modeling Methodology
 Selection of Watersheds
 Selection of Variables
 Dependent Variables: Low Flow Discharge Rates
 Independent Variables: Watershed Characteristics
 Data Collection
 Application of the Modeling Methodology
 Discussion of Results
- IV. Conclusions
- V. Application of the Model
- VI. Bibliography
- Appendix A Flow Duration Curves Low Flow Frequency Curves
- Appendix B Discharge Data: USGS WATSTORE File

I. INTRODUCTION

PURPOSE OF STUDY

Information on low flows is useful for a wide range of planning, design and management applications in water supply, hydroelectric power generation, fish and wildlife protection, cooling water and effluent dilution, navigation, recreation, and the preservation of aesthetic features of lakes and streams. With the rapid growth of population in New Hampshire, the demands for low flow information have become imperative. The purpose of this study is to investigate the magnitude-frequency relationships of naturally occuring low flows on streams in New Hampshire to facilitate decision making in the management of water resources in the state.

SCOPE OF STUDY

The scope of work for this project included the following tasks:

- Conduct a literature search to establish the availability of any regional low flow frequency studies that may have been performed for the State of New Hampshire;
- 2) Identify existing water resource projects affecting low flows in the respective river basins; ascertain pertinent data on these projects and present operating rules and/or procedures; assess the probable effect of regulation from existing projects on downstream flow;

3) Develop a simple procedure for estimating low flow for natural (unregulated), ungaged streams in New Hampshire.

In this report, past studies on low flow rates are reviewed. The development of a procedure to estimate low flows in ungaged watersheds in New Hampshire, and the statistical testing of the procedure are described, and a brief list of conclusions is provided. Finally, the steps for application of the procedure are outlined. The appendices contain low flow frequency curves and flow duration curves for the 30 watersheds examined in the study, and selected low flow data from the U.S. Geological Survey's WATSTORE file for 65 gages. The set of 65 gages is comprised of all New Hampshire gages, as well as those gages in Maine and Vermont that were included in the statistical analyses for this study.

II. LITERATURE REVIEW

INTRODUCTION

The State of New Hampshire has clearly recognized the need for information on low flow discharge rates of streams and for a methodology to estimate the low flows of various durations in ungaged watersheds. In many other areas of the nation, there is a similar need. Recently, the Task Committee on Low Flow Evaluation, Methods and Needs, of the Committee on Surface Water Hydrology of the Hydraulics Division of the American Society of Civil Engineers, conducted an investigation with the following objectives: to determine types of low flow information needed for typical problems; to describe various low flow characteristics as well as methods and data needed for defining those characteristics and their accuracy; and to suggest further analyses and data collection needs. During its investigation, the Task Committee found that, (1) methods are available for defining low flow characteristics from flow records and from base flow measurements: (2) methods for estimating low flow characteristics at sites without any flow data are inadequate for certain purposes; and (3) the use of standardized methods for defining various low flow characteristics such as the annual m-day low flow or the minimum flow of record, is not possible yet because variations in the amount and reliability of data at different sites require that different methods be used depending on the data available. This Committee's study did not include an investigation of the effects of basin factors on low flows because it believed

that those effects are "difficult to describe quantitatively and therefore are of little value in estimating low flow characteristics" (Task Committee, 1980, p. 717). Despite the beliefs of the Task Committee, our literature review revealed that several investigators have achieved some degree of success in analyzing the relationships among low flow discharge rates and watershed and climatic parameters. A limited number of these studies have been conducted in northeastern United States. At the outset of the present investigation, these past studies were reviewed for information on methodology, variables considered in the analyses, and findings. A brief discussion of these studies is outlined below.

RESEARCH METHODOLOGIES

Flow Duration Curves

As defined by Searcy (1959, p. 1), the flow duration curve is "a cumulative frequency curve that shows the percent of time specified discharges were equalled or exceeded during a given period. It combines in one curve the flow characteristics of a stream throughout the range of discharge without regard to the sequence of occurrence." Flow duration curves are useful in comparing one watershed to another.

The slope of the lower end of a flow duration curve indicates the storage characteristics of a watershed: a flat slope indicates a watershed with a relatively large amount of water in storage and thus relatively greater low flow rates, while a steep slope

indicates a watershed with less water in storage and thus relatively lesser low flow rates. Estimates of flow duration curves for sites on ungaged streams can be made by establishing a relation between that stream and nearby gaged streams through plots of concurrent baseflow discharge measurements in the streams.

A flow duration curve is therefore a summary of all daily flows for the period on which it is based, with no regard to sequence. In contrast, low flow frequencies, which are based on annual low flows for a specified number of consecutive days, are more related to the occurrence of drought (Hely and Olmsted, 1963). Low flow frequency characteristics of ungaged streams can be estimated from regional relations of discharge to drainage area in regions homogeneous with respect to geology, topography and climate. The relations are developed from concurrent baseflow measurements on gaged streams over time. Identification of seepage runs, which consist of measuring discharge along a channel reach during a period of base flow, can further refine estimates of flows at ungaged sites along a stream (Riggs, 1972). In each of these methods, the influence of watershed characteristics on low flows is recognized.

Watershed Characteristics and Low Flows

The relationship between the physical characteristics of a watershed and stream flow has been examined by investigators using a variety of methodologies. The factors being studied determine which method of analysis best applies. The principal natural factors, which are examined individually and in combinations as influences on

streamflow, include watershed size, precipitation, surficial geology, elevation, vegetation, and slope. Recent investigations of watersheds in New England (e.g., Dingman, 1978; Tasker, 1972; Comer and Zimmerman, 1969; Comer and Dunne, 1968; Thomas, 1966) have found elevation and surficial geology to be useful in estimating low flow discharge rates.

Low flow discharges are obviously related to precipitation, as it is the water input to the physical system of the watershed. The relationship between precipitation and low flow discharge rates, however, is an indirect one. The volume of precipitation that eventually enters a stream channel during a low flow period is mediated through several factors: climate, which determines the evapotranspiration rates, as well as the amount and form of the precipitation; the geology of a catchment, which determines infiltration and groundwater flow rates; the topography of the watershed, which affects rates of runoff and infiltration; and land use and vegetative cover which influence rates of runoff, infiltration, evaporation, and transpiration.

Various precipitation factors have been investigated as indicators of low flow discharge rates. Thomas and Benson (1975) studied the relationship between streamflow and several watershed characteristics in four regions of the United States. They found that three low flow parameters ... are significantly related to mean annual precipitation but not to precipitation intensity (24 hr, 2 yr) in the eastern region. However, they cautioned that the relations which they defined for low flow estimates are unreliable

and can provide only rough estimates of low flow discharge rates at ungaged sites. In contrast, Lull and Sopper (1966) found that mean daily discharges of the 90% flow duration were correlated with precipitation intensity (24 hr, 2 yr) slightly more than with average annual precipitation at the nearest weather station. However, their regression equation is only able to explain 23% of the variance in that low flow parameter.

Other investigators have examined the role of precipitation as an influence on low flow in greater detail. Comer and Dunne (1968) developed a precipitation index by using recession constants of streams in northeastern Vermont to weight daily rainfall amounts before and during the times of minimum streamflows. This precipitation index is then multiplied by a factor determined from either drainage density (for watersheds underlain by poorly drained soils), or stream frequency (for watersheds underlain by well-drained soils) to predict minimum streamflows. Correlation of the precipitation index with the minimum streamflow volumes yielded coefficients of determination (R²) ranging from 0.87 to 0.97.

Chang and Boyer (1977) used estimates of the September 10-year maximum consecutive rainless days and the September mean 7-day 10-year maximum daily temperature as climatic parameters in an investigation of low flows on 12 Monongahela tributaries in West Virginia. These climatic parameters were used in the analysis as indices of the evaporative power and drought potential of the low flow period. This study also demonstrates the value of using a precipitation factor which has a clearly defined and logical

relation to low flows; the regression equation developed in this investigation is able to explain 99% of the variance in the minimum 7-day 10-year flow. In general, those investigators who have attempted to select variables which are representative of the physical processes of runoff, infiltration and evapotranspiration have been more successful in using a precipitation parameter to estimate low flows.

In many watersheds in New England, streamflow during the summer and early autumn consists primarily of groundwater flow. This fact suggests that the geology of a watershed may be an important influence on low flow discharge rates. As described earlier, Searcy (1959) demonstrated that estimates of flow duration curves at sites on ungaged streams can be made by establishing a relation between that stream and nearby gaged streams through plots of concurrent baseflow discharge measurements on the streams. Tasker (1972) developed a method for estimating low-flow characteristics of ungaged streams in southeastern Massachusetts from basic geologic factors and ground water availability maps. He defines a "groundwater factor" as a rough approximation of the average transmissivity, in hundreds of gallons per day per foot, of the watershed. Through regression analysis, the relationships between the 7-day mean low flow at the 2- and 10-year recurrence intervals and the two independent variables, drainage area and the groundwater factor, are defined.

Hely and Olmsted (1963) believe that the geologic character of a drainage basin is the "terrestrial factor" most influential on

streamflow. They demonstrated the close relationship between lithology and low flow in 19 watersheds in northeastern United States. Average values of $\mathbb{Q}_{90}/\mathbb{Q}_a$ (where \mathbb{Q}_{90} = the discharge equalled or exceeded 90 percent of time; and \mathbb{Q}_a = the average annual discharge) were determined. For each watershed, the area underlain by each geologic formation was weighted according to these average values of $\mathbb{Q}_{90}/\mathbb{Q}_a$. The correlation coefficient between the estimates of low flow based on geology and the actual low flow for the sub-basins was 0.9.

Thomas (1966) found a direct relationship between surficial geology and low flow in 23 watersheds in Connecticut. Flow duration curves for these watersheds show that the "median annual minimum 7-day average flow ... is 1.30 cfs per square mile from a drainage area underlain exclusively by stratified drift, and only 0.013 cfs per square mile from an area underlain exclusively by till, a ratio of 100 to 1" (Thomas, 1966, p. B209). Streams in the Susquehanna River basin exhibit the same relationship between surficial geology and low flow values (Ku, Randall, and McNish, 1975).

Cross (1949) compared flow duration curves of several watersheds in Ohio and attributed the differences in shape to variations in the geologic characteristics among the watersheds. He concluded that it is valid to predict the geologic characteristics of a watershed from an analysis of its flow duration curve, but that the converse is not a valid approach.

A study by Comer and Zimmerman (1969) of two streams in northern Vermont attributed differences in discharge rates (of flow durations ranging from 1 to 30 days) to differences in topography and soils. Flow duration curves for the two small (3.2 and 8.4 square miles) watersheds were compared. Since the climate, geology and land use are similar, the infiltration and drainage characteristics of the soil types in the two watersheds are believed to be the major influence on the low flow rates of the streams.

Ives (1977) conducted an empirical analysis of flow duration curves in an attempt to define a method for estimating discharge rates in ungaged watersheds in New Hampshire. No significant correlation was found between two baseflow parameters ($\mathbb{Q}_{90}/\mathbb{Q}_{a}$ and $\mathbb{Q}_{98}/\mathbb{Q}_{a}$) and watershed size for the 14 watersheds in New Hampshire. An attempt to relate low flow discharge rates and surficial geology produced unsatisfactory results due to lack of detailed soils or geologic data. Regional groupings of watersheds reflect variations in mean annual runoff; however, this information does not aid in the prediction of flow distribution in an ungaged watershed.

In contrast, Dingman (1978) investigated 53 watersheds in New Hampshire and found that estimated mean watershed elevation is significantly correlated with the ratio of flow exceeded 95% of the time to drainage area Q_{95}/A_D (ft $^3/\text{mi}^2$). He explains that the importance of elevation as an influence on low flow rates is due to lower temperatures and evapotranspiration rates, as well as greater and longer-lasting snowpack at higher elevations. Thus, these factors hold moisture in the watershed and result in a more even streamflow throughout the year. Flow duration curves for

ungaged streams can be approximated by calculating values for the 2%, 5%, 30% and 95% durations using equations developed in this article, and then plotting these values on log-probablility paper and drawing a smooth curve through the points. Calculation of the 95% confidence limits for these flow duration curves is also explained. In this investigation, mean watershed elevation is estimated by a regression equation developed from data for watersheds in New Hampshire. Dingman states that the use of measured mean watershed elevation can significantly reduce the confidence intervals about the estimated values.

The relationship between climate and elevation was also recognized by Chang and Boyer (1977) in their investigation in West Virginia. They used regressions of elevation and latitude with temperature and precipitation parameters at weather stations in the area, to estimate values of those climatic parameters in the watersheds being studied. They pointed out the high correlation between elevation and latitude and the temperature and precipitation factors.

Other investigators have attempted to determine the relative influence of several watershed characteristics on streamflow rates. Lull and Sopper (1966) examined the role of 14 independent variables as influences on seasonal average dischage rates as well as on various flow durations in 137 small (less than 100 square miles) watersheds in the northeastern United States. They found that the most influential basin characteristics are precipitation, percent of the watershed in forest cover, elevation, latitude, average July

maximum temperature, and the percent of watershed area that is swamp. However, none of the factors is highly correlated with the 90% duration flow.

Studies which examine the relationship between vegetation and low flow discharge rates have yielded contradictory results. studies, forest cover is found to be inversely related to streamflow in watersheds in New England and New York (Schneider and Ayer, 1961; Lull and Reinhart, 1967). These studies were conducted in experimental watersheds and consisted of monitoring streamflow in forested catchments before and after clearcutting the basins. each case, streamflow increased after the trees were cut; the increases in discharges were attributed to decreased evapotranspiration losses. In contrast, Lull and Sopper (1966) found a positive correlation between runoff and the proportion of a watershed in forest. The authors suggest that forested land may integrate a number of factors, such as slope, latitude, temperature and soils, which produce greater runoff. In addition, they emphasize the likelihood that the yield of streams in these watersheds would increase if the trees were cut and the evapotranspiration losses decreased.

Few studies of low flows in northeastern United States have identified slope as an important influence on low flow discharge rates. Valley slope (change in valley bottom altitude over the same valley reach divided by valley length), was found to be significant in a regression equation for the 7-day two year low flow, but not in the equation for the 7-day ten year low flow (Ku, Randall, and

McNish, 1975). Lull and Sopper (1966) found a correlation coefficient of less than ± 0.316 between main channel slope and average annual and average seasonal (four seasons) runoff.

Other natural characteristics of watersheds have been examined in attempts to determine their relationship to streamflow. Examples of these are latitude, aspect, average July maximum temperature, percentage of swamp in the basin (Lull and Sopper, 1966); valley length, sinuousity, valley slope, length²/area, solar radiation factor (Ku, Randall and McNish, 1975) and stream density (Hely and Olmsted, 1963).

CONCLUSIONS

The relationship between the precipitation that falls on a watershed and the discharge of the stream draining is an indirect one. Natural characteristics of the catchment influence the runoff, infiltration, and evapotranspiration processes which operate in the watershed. The influences of natural factors on streamflow have been studied in experimental and natural watersheds. In northeastern United States, low flows are found to be positively correlated with elevation in New Hampshire and the percentage of the watershed underlain by stratified drift in Connecticut and inversely related to vegetative cover in both New England and New York. To effectively manage the water resources in New Hampshire, an accurate method for predicting low flow discharge rates in ungaged watersheds would be desirable. Definition of the relationships between watershed characteristics and streamflow is a necessary first step.

III. REGIONAL ANALYSIS OF LOW FLOW DISCHARGE RATES

DESCRIPTION OF PHYSICAL ENVIRONMENT Geomorphology

New Hampshire can be broken into three general classes of landforms: the coastal plain, the lowlands and foot hills, and the mountains. The southeastern corner of the state is mainly coastal plain, the center of the state is mainly lowlands/foothills and the northern portion of the state is mountainous. There is a gradual increase in elevation and surface irregularity proceeding from the south to the north. Unconsolidated deposits in the mountainous region are predominantly composed of thin layers of till with pockets of stratified drift. The areas of stratified drift, which consists of sands, gravels, and clays, are found in the valleys of the major streams. The lowland/foothills region is characterized by somewhat thicker layers of till on the slopes with more frequent and larger areal distributions of stratified drift along the region's streams. Deposits of marine clays and sands, interspersed with till, which are found in the coastal plain, indicate that the area was below sea level at one time.

Climate

Two factors in the state geography control major aspects of the regional climate: topography and proximity to the ocean. Topography appears to play a major, complex role, affecting temperature, precipitation, moisture storage, and stream discharges. Moist air masses tend to enter the state from the southeast, south, southwest

and west. As they pass through the state towards the east, northeast and north, elevation changes in the ground surface force the air mass aloft, causing cooling by decompression, and precipitation. The areas of highest mean annual precipitation are in the northern portion of the state.

Topography also affects the mean annual temperature within the state. Increases in elevation cause a gradual drop in mean temperature (about 3.5° F per 1000 ft. in elevation). Therefore, the higher elevations tend to be colder, to have a greater portion of their precipitation as snow, and to have snowmelt discharges throughout greater portions of the year.

Proximity to the ocean has a moderating effect on the temperature variations throughout the year. Sites close to the ocean tend toward milder winters and summers, while inland sites tend toward colder winters and warmer summers.

GENERAL PROCESSES THAT CONTROL WATERSHED LOW FLOWS

To predict low flow, it is necessary first to understand the basics of the operating system that produce low flow. Moisture from precipitation flows directly to the stream or surface detention areas, such as lakes and ponds, or infiltrates the ground surface. The infiltrated moisture is capable of flowing through the soil and downslope to the nearest stream, but will generally do so only when the soil moisture storage has exceeded its holding capacity. Since soil moisture is withdrawn through the evapotranspiration process, soils tend to be drier during warmer seasons of the year.

Therefore, less water is free for streamflow during periods of higher evapotranspiration, and precipitation plays a seasonally changing role in stream water supply throughout the year.

The relationship between precipitaton and streamflow is further confounded by the movement of water through the ground. The potential volume of water which accumulates in and moves through the soil and subsurface layers is directly related to the type of geologic material which comprises these layers. Porous materials, such as sands and gravels, can collect and store large amounts of water and release it freely to streams. Flow of water from these porous layers to stream channels augments streamflow during times of drought. Watersheds drained by porous aquifers generally have more moderate flows during extreme wet and dry periods. In contrast. solid bedrock and fine-grained sediments, such as clays, permit minimal infiltration and movement of water through the subsurface layers. Runoff in these watersheds flows directly over the ground surface to the stream channels producing a "flashy" stream. there is limited potential for groundwater contribution to streamflow during times of drought, these streams tend to have extreme low, as well as high flows. The watershed geology and its interrelationships with other geomorphic characteristics of a watershed, such as elevation, slope, and surface storage, therefore, influence the severity of any climatically-induced low flow event.

LOW FLOW MODELING

Selection of Modeling Methodology

One objective of this study is to provide a procedure that can be used to estimate low flows in ungaged watersheds with limited data on physical parameters. This implies that the procedure must be developed from available data and from an understanding of the systems operating within these watersheds.

Since the study will be applied to watersheds with limited data, simulation models can probably be eliminated as a viable approach. Measurements of baseflow and investigation of seepage runs were also determined to be beyond the limits of the scope of work for this study although several sources had advocated concurrent baseflow measurements for specific predictions of low flows in ungaged watersheds. The problem can be dealt with through some form of a statistical model. Statistical models (regression, factor analysis) allow the researcher to quantify the relationships between key controlling variables and a dependent variable. In this case, the objective is to relate certain climatic and watershed geomorphologic variables to low flow rates.

Application of statistical models creates some significant temporal and spatial issues that must be resolved before the model is developed. These models are based on a sample of watersheds with known flow, climatic, and geomorphologic data. These data are then used to explain the interwatershed variation in flow, or in this case, low flow. The independent variables used (climate and geomorphology) must reflect both the spatial and temporal changes in

conditions that influence low flow. For example, the amount of precipitation will be critical in determining peak and low flows for any given watershed, but the amount of precipitation will vary from watershed to watershed (spatial variation). Also, the distribution of precipitation throughout a single year (temporal variation) may vary from watershed to watershed. Two watersheds with similar annual rainfall may have significantly different dry season precipitation, resulting in significantly different low flows. The selection of variables should reflect a functional relationship with the spatial and temporal variations in low flow controls.

Once these variables have been selected and quantified, they can be related to low flows through the use of linear or non-linear regression techniques. These techniques will use interwatershed variations in both the independent (climatic and geomorphologic) and dependent (low flow) variables to create an equation that expresses the relationship between the two. Once the equation has been computed and tested, it can be used to estimate low flow events in ungaged watersheds.

Selection of Watersheds

The statistical analyses performed in this study and the resulting regression equations utilized streamflow data obtained from U.S. Geological Survey (USGS) stream gaging stations located in New Hampshire or in close geographic proximity to New Hampshire.

The intended use of these equations is to estimate low flow values for "natural" conditions on New Hampshire streams. Therefore, the

USGS gages selected for use in the regional analysis were determined to have streamflow data representative of natural low flow conditions. The equations were developed and tested using data obtained from 30 USGS gaging stations located in New Hampshire, Vermont and Maine. Average and minimum low flow data (in cubic feet per second per square mile, or csm) for the 1-, 7-, 30-, 90-, 183-, and 365-day durations for the period of record for each of the 30 gages are given in Table 1.

The gages utilized in the regional analysis were selected using the following procedure. As stated in the scope of work, gages first had to meet two criteria: 1) have a contributing drainage area of 10 square miles or greater; and 2) possess 15 years or more of continuous records. A list of gages was compiled using recent USGS Water Resource Data reports for New Hampshire, Vermont, Maine and Massachusetts. Several discontinued gages which did not appear in the recent USGS Water Resources Data reports were added to the list. The final list of stream gages which met the criteria for drainage area size and length of continuous record included 89 gages located in New Hampshire, Vermont, Maine, and Massachusetts. Fifty-four of the gages fall within the boundaries of New Hampshire.

The 89 gages were further investigated to determine which ones provided data representative of natural low flow conditions. The initial step of this investigation determined which streamflow records were affected by regulation using the information contained in recent USGS Water Resources Data reports. Generally, the most pertinent information contained in these reports was included in the

TABLE 1.
HISTORIC FLOW DATA: GAGES USED IN REGIONAL ANALYSIS

Basin USGS ID No.	Description	Period of Record	. Ad	± ō	ean Ann Follo	Hean Annual Low Flow (csm) For Following Durations (days)	Flow (ca	sm) days)		For	Minimum Low Flow (csm) of Record For Following Durations in Days	Flow (csm) of	Record n Days	
			(m1 ²)		7	30	90	183	365	_	. 2	30	90	183	365
Andros coggin River															
01052500	Diamond R. nr Wentworth Location, NH	1943-1978	153		.2178	.1805 .2178 .3355		1.1852	.6073 1.1852 2.3036	4440.		.0588 .1438	.1307	.4052	.1307 .4052 1.6797
01054300	Andover, ME	1964-1979	131	.1727	.1727 .1880	.2562	.4322	.9795	9795 1.9504	9160.	.0992	.1374	.2214	.3282	1.1069
	nr South Paris, ME	1915-1979	76.2	.0965	7811. 2960.	.2064	.3903	.7860	1.8377	.0131	.0131	.0289	.0787	1444	8136
Saco River															
01064300	Ellis R. nr Jackson,														
01064500	Saco R. nr Conway, NH	1965-1979 1905-1978	10.9 386	. 5058	.5346	.4902	1.0232	1.8440	3.0887	.2477	.1917	.2844	. 4679	.8716	2.2018
Piscataqua River			,												
01073000	Oyster R. nr Durham, '	0701-3201		00 70	6			. !				į			
Merrimack River	ı	כוכן הככי			56/0. 0000.	5611.	02120	4959	.4959 1.6037	.0289	.0289 .0355 .0454 .0603	.0454		6060	. 7273
01074500	E.Branch Pemigewasset														
01075000	R. nr Lincoln, NH Pemioewasset R. at	1930-1953	104	4066	.4066 .4471	. 5549	.8658	1.5324	.8658 1.5324 2.8946	.125	.125	.1538 .25	.25	.4038 2.0577	2.0577
01076000		1941-1921	193	.3674	.3674 .4040	.5260	.8331	1.4870	2.6802	.2176	.2228 .2487	.2487	.3316	.3990 1.8446	1.8446
01076500	NH Peminewasset R. pr	1930-1977	143	.1541	.1541 .1700	.2469	.4270	.8148	.8148 1.7851	6920.	.0839	.1119 .1538		.2308 1.0350	1.0350
	Plymouth, NH	1905-1978	622	.2660	.2660 .3033 .4057	.4057	.6310	1.1292	.6310 1.1292 2.1884	.0723	.0723 .1061 .1592 .1849	.1592	.1849	.3440 1.3874	1.3874

TABLE 1 (Continued)

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•			ייים ייים ובסיים הייים ועיים		משפנים מ	GAGES USED IN REGIONAL ANALYSIS	AEGI UNA	L ANALY	SIS		•				
Basin USGS 1D No.	Description	Period of Record	PA	¥ [0]	Follow	Mean Annual Low Flow (csm) For Following Durations (da	Flow (ations	(days)	d.	Σ-	Minimum Low Flow (csm) of Record For Following Durations in Days	Low Flor	w (csm) Juration	of Reco	brd ys
1			(mi2)	-	7	30	90	183	365	_	7	30	8	183	365
Merrimack River (continued)															
01078000	Smith R. nr Bristol,	1920-1979	85.8	.1092	.1343	9961.	.3331	.6521	1.6657	.0315	.0373	.0524	1014	.1865	.8275
01084500	Beards Br. nr Hillsboro, NH Warner R. nr Davisville	1947-1970	55.4	.0512	.0627	.1126	.2255	.5356	1.6464	4500.	.0085	.0179		4190.	.7581
01087000	NH Blackwater R. nr Webster.	1941-1978	971	.0790	4460	.1321	.2432	.5698	1.6386	.0192	.0226	.0253	.0411	11164	.7808
01089000	NH Soucook R. ar Concord.	1920-1979	129	.1766	8461.	.2457	.3814	.6768	1.6462	.0589	1590.	8690.	.1085	.1938	. 7984
01091000	NH S.Branch Piscataquoq R.	1953-1980	76.8	.0792	7680.	.1296	.2223	.4972	1.4253	.0299	.0338	.0456	.0547	.1185	.7161
01094000	nr Goffstown, NH Souhegan R. at Merrimack,	1942-1978	104	.0822	7960.	1484	.2618	.5834	1.5896	.0231	.0279	.0413	.0548	.125	.7308
	. HN	1911-1976	171	.1431	.1682	.2235	.3390	.6316	1.6689	.0234	.0281	.0415	09/0	1404	.7368
Connecticut River															
01130000	Upper Ammonoosuc R. nr	0001-0101	9												
01133000	E.Branch Passumpsic R.	1347-1300	757	.2754	.3118	.4125	.6341	.6341 1.0575	2.1001	.1379	.1595	.2026	.2543	.3405	1.444
01134500	nr East Haven, VT Moose R. nr Victory, VT Moose R. nr St. Johnsbury.	1941-1979 1948-1979	53.8	.4007	.1449	.6063	. 8550	1.2207	1.9817	.2416	.2602	4461	.6134	.7249	1.3197
01137500	Ammonoosuc R. pr	1930-1979	128	.1136	.1382	.2408	.4267	.8223	1.7481	.05	.0578	1094	.1953	.2344	1.1484
	Bethlehem Jct, NH	1941-192	87.6	.3849	.4165	.5266	. 7912	.7912 1.3356	2.3932	.2397	.2740	.3196	.4110	.5023	1.6667

TABLE 1 (Continued)

HISTORIC FLOW DATA: GAGES USED IN REGIONAL ANALYSIS

Basin		Period			For F	Mean Annual Low Flow (csm) For Following Durations (days)	Low Fl	ow (csm) (8/1		finimum For Fol	Minimum Low Flow (csm) of Record For Following Durations in Days	W (csm)	of Rec	ord
USGS ID No.	Description	of Record	DA (m1 ²)	-	7	30	90	183	365		7	Ş	8	183	365
Connecticut River (Continued)											-	3	3	6	200
01138000	Amonoosuc R. nr Bath,														
01142500	NH Ayers Br. at Randolph,	1937-1979	395	.1965.	.2392	.3302	.5054	.8904	1.6545	.0886	.1392	.1620	.2202	.3190	1.0658
01145000	VT Mascoma R. at W. Canaan,	1941-1978	30.5	.1337	.1545	.2207	.3561	.6307	1.5164	.0262	.0318	.0492	.0787	.2197	.8197
01.153500	NH Williams R. at Brockaway	1941-1978	80.5	.0958	.1127	.1828	.3349	.6358	1.4763	.0373	2440.	.0546	.0882	.1366	.7950
01154000	Mills, VT Saxtons R. at Saxtons	1942-1979	103	.1084	.1212	1798	.3163	.6254	1.6957	.0350	.0408	.0709	1068	.1748	.8544
01155000	River, VT Cold R. at Drewsville,	1942-1979	77.2	.0938	.1080	.1650	.2900	.5845	1.5748	.0311	.0389	.0570	0620.	.1140	.6995
01156000	NH West R. at Newfane, VT S.Branch Ashuelot R.	1942-1978 1930-1961	82.7 308	.0956	.0964	.1472	.2651	.5380	1.4324	.0230	.0487	.0423	.0629	.0871	.6046
	nr Mariborough, NH	1922-1978	36.0	.0810	.1220 .1936		.3583	.6714	1.6535	.0111	.0139	.0528	9080.	.1333	.6667

station description under the category entitled "Remarks". The data contained in this category included information pertaining to upstream projects that affect natural flows at each gaging station.

For each gage affected by some form of regulation or diversion, a list of the flow-affecting upstream projects was developed.

Because the recent USGS Water Resources Data reports did not provide any data relevant to discontinued gages, information on the discontinued gages was obtained using USGS Water Supply Papers which were compilations of all past gage data (U.S. Department of the Interior, 1954, 1964). These compilations contained station description data similar in nature to the data found in the recent Water Resources Data reports. Information pertaining to upstream projects affecting the discharge data at each discontinued gage was obtained from this source. Using information compiled from the USGS publications, it was determined that streamflow data at 72 of the 89 gages were affected by regulation to some degree.

The next step in the process of selecting gages representative of natural low flow conditions involved simultaneous data collections from two sources, the New Hampshire Water Resources Board (NHWRB) and utility companies, to obtain detailed information regarding major water projects, such as recreational reservoirs and storage pools for hydroelectric generating facilities, which affect low flow data. (see Table 2).

The goal of this effort was to determine if sufficient data were available to quantify the effects of regulation and enable natural low flow conditions to be calculated. The results of these

TABLE 2

MAJOR WATER PROJECTS WITH SIGNIFICANT EFFECT ON LOW FLOWS FOR NEW HAMPSHIRE STREAMS

		Project Name	Type of Project	Drainage Area (square miles)	Usable Storage (acre-feet)	Remarks
	Andr	Androscoggin River Basin				
	<u>-</u> :	l. Errol Dam	Hydroelectric	1,045	643,000 ²	An agreement exists to provide a minimum flow of 1500 cfs downstream of Errol Dam.
	Conn	Connecticut River Basin				
	2.	Moore Dam	Hydroelectric	1,600	114,176	
	3	3. Comeford Dam	Hydroelectric	1,635	32,270	
2	4.	McIndoes Falls Dam	Hydroelectric	2,200	5,000	
4	7.	Wilder Dam	Hydroelectric	3,375	13,350	Minimum flow equal to 0.2 cubic feet per second per square mile (csm)
	9	Bellows Falls Dam	Hydroelectric	5,414	9,568	Minimum flow equal to 0.2 csm
	7.	Vernon Dam	Hydroelectric	6,266	18,300	Minimum flow equal to 0.2 csm
	∞	First Connecticut Lake	Storage Pool & Recreational Lake	45	11,600	
	9	Second Connecticut Lake	Storage Pool & Recreational Lake	82	76,400	

l Information obtained from the Water Quality Management Plans published by NHWSPCC and from data provided by the various utility companies.

² Usable upstream storage figure includes storage contained in Rangley Lake, Mooselookmeguntic Lake, Upper and Lower Richardson Lakes, Aziscohos Lake and Umbagog Lake.

Project Name	Type of Project	Drainage Area (square miles)	Usable Storage (acre-feet)	Remarks
Connecticut River Basin (continued)	:inued)			
10. Lake Francis Dam	Storage Pool & Recreational Lake	170	99,300	
ll. Mascoma Lake Dam	Recreational Lake	182	24,400	
12. Lake Sunapee Dam	Recreational Lake	94	19,800	
derrimack River Basin				
13. Ayers Island Dam	Hydroelectric	947	not given	
l4. Eastman Falls Dam	Hydroelectric	1,003	1,200	
15. Garvin Falls Dam	Hydroelectric	not given	not given	
16. Hooksett Dam	Hydroelectric	not given	not given	
17. Amoskeag Dam	Hydroelectric	not given	not given	
18. Jackman Dam	Hydroelectric	69	4,040	
19. Squam & Little Squam Lakes Dam	Recreational Lake	58	46,000	
20. Newfound Lake Dam	Recreational Lake	96	25,000	
21. Lake Wentworth	Recreational Lake	35	19,600	
22. Merrymeeting Lake Dam	Recreational Lake	Ξ	8,400	
23. Lake Winnipesaukee Dam	Recreational Lake	363	165,500	
iscataqua River Basin				
24. Great East Lake Dam	Recreational Lake	12	11,800	

TABLE 2 (continued)

Remarks

Usable Storage (acre-feet)		13,300	9,400	3,500	11,480	23,050
Usable (acre		13	6	60	=	23
Drainage Area (square miles)		105	20.66	5.4	not given	330
Type of Project	(continued)	Recreational Lake	Recreational Lake	Recreational Lake	Recreational Lake	Recreational Lake
Project Name	Piscataqua River Basin (co	25. Milton Pond Dam	26. Pawtuckaway Lake	27. Mendums Pond	28. Conway Lake	29. Ossipee Lake

investigations determined that the data were not sufficient to compute natural low flow values at the gages located downstream of these projects. The 49 gages which came under this classification were deleted from consideration in the regional analysis.

The next stage in the selection process was to investigate all gages which, according to the USGS records, were subject to minor regulation by various water resources projects. "Some regulation," "some diurnal fluctations," and "slight regulation" are examples of the descriptions found in the USGS reports. Often the regulation was by unnamed mills or ponds. Twenty-two gages were found which fell into this classification. Further investigation of these gages was conducted using information found in the files maintained by the NHWRB. Because there are a large number of dams on New Hampshire streams it was deemed infeasible to review the files of every dam located upstream of each gage under consideration; therefore, only storage dams with pools large enough to appear on USGS topographic quadrangle maps were considered to have a significant effect on natural low flow conditions. The assumption that storage pools which are too small to be detected on USGS quad sheets have an insignificant effect on low flow values, appears to be reasonably valid, especially when analyzing low flow for durations of 7 days or greater. On the basis of this assumption, the streamflow data at 9 of the 22 gages were judged to be significantly affected by upstream regulation and not representative of natural conditions. Further investigation of the available information contained in the NHWRB files also indicated that natural streamflow values could not be

calculated given the existing data base. Therefore, these 9 gages were not included in the regional analysis. The remaining 13 of the 22 gages which are subject to regulation by minor water projects were deemed to be representative of natural low flow conditions and were therefore included in the regional analysis.

Finally, flow duration curves were plotted for the remaining 31 gages. A comparision of the general shapes of these flow duration curves indicated one watershed (located in Massachusetts) which was judged to be unrepresentative and was thus deleted from consideration in the regional analysis. Flow duration curves for the 30 watersheds in the regional analysis comprise Appendix A. Also contained in Appendix A are low flow frequency curves for the 7-, 30-, 90-, and 183-day durations for each of the 30 gages.

Comprising the final list of 30 gages with streamflow data determined to be reflective of natural low flow conditions were 17 gages which are not regulated by any projects and 13 gages which are affected by insignificant amounts of regulation. A final map check was made by comparing the locations of the 30 gages with the locations of all significant municipal water projects (wastewater treatment plants and water supply intake facilities) in New Hampshire (see Tables 3 and 4).

Wastewater treatment plants were considered significant if they had a permit to discharge 1.0 cfs (0.65 mgd) or more. In order to comply with water quality standards, the majority of the wastewater treatment plants discharge effluent into large streams which provide substantial assimilative capacity. Therefore it was assumed that

TABLE 3

SIGNIFICANT* MUNICIPAL WATER PROJECTS - WASTEWATER TREATMENT PLANTS

RECIPIENT STREAM		Androscoggin River		Ammonoosuc River		Sugar River	Sugar River	Sugar River	Sugar River		Connecticut River	Ashuelot River		Squam River	Merrimack River
PERMITTED DISCHARGE (CFS)		4.1		1.76	2.30	1.67	0.65	2.02	1.00	6.40	1.10	5.00		1.60	11.5
PERMI (CFS)		4.9		2.73	3.56	2.59	1.00	1.30	1.55	9.92	1.70	7.75		2.48	17.82
COMMUNITY	Androscoggin River Basin	Berlin	Connecticut River Basin	Littleton ²	Hanover ²	Lebanon	Sunapee	Newport (Primary Plant) ²	Newport (Dorr Woolen)	${\tt Claremont}^2$	Charlestown	Keene	Merrimack River Basin	Ashland	Franklin (WRM)

All treatment plants with discharge greater or equal to 1 cfs (0.65 mgd)

Information obtained from State of New Hampshire National Water Quality Inventory published by WSPCC Not completed as of April 1980 *- 0

TABLE 3 (continued)

SIGNIFICANT* MUNICIPAL WATER PROJECTS - WASTEWATER TREATMENT PLANTS

COMMUNITY	PERMITTED DISCHARGE (MC)	DISCHARGE (MGD)	RECIPIENT STREAM
Merrimack River Basin (continued)	(pen)		
Concord (Penacook)	6.51	4.2	Merrimack River
Concord (Bow) ²	15.65	10.1	Merrimack River
Allenstown-Pembroke	1.63	1.05	Merrimack River
Manchester	40.3	26.0	Merrimack River
Milford ²	3.33	2.15	Souhegan River
Merrimack	7.75	5.0	Merrimack River
Nashua ²	33.32	21.5	Merrimack River
Salem	3.78	2.44	Spicket River
Piscataqua River and Coastal	NH Basin		
Somersworth	3.74	2.41	Salmon Falls River
Rochester	60.9	3.93	Cocheco River
Dover -1^2	6.67	4.3	Cocheco River
Durham ²	3.88	2.5	Oyster River (tidal)
Exeter ²	4.05	2.61	Squamscott River
Hampton	7.28	4.7	Mill Creek (tidal)
Pease Air Force	1.86	1.2	Piscataqua River (tidal
Portsmouth ²	7.18	4.63	Piscataqua River (tidal

TABLE 4

SIGNIFICANT* MUNICIPAL WATER PROJECTS - WATER SUPPLY FACILITIES

WATERSHED AREA (mi ²) DOWNSTREAM BODY OF WATER		not given Androscoggin River	not given Upper Ammonoosuc River	8.37 Androscoggin River		9.25 Ammonoosuc River	30 Mascoma River	30 Ammonoosuc River	not given Connecticut River	5.0 Sugar River	12.0 Mascoma River	40.85 Ammonoosuc River	8.0 Otter Brook	
SOURCE		Androscoggin River	Upper Ammonoosuc River	Ice Gulch and Perkins Brook		Gale River & Zealand River	Canaan Street Lake	Little River	Beaver Brook	White Water & Granby Brook	Harris Brook	Ammonoosuc River	Babbige Reservoir	
COMMUNITY	Androscoggin River Basin	Berlin	Berlin	Gorham	Connecticut River Basin	Bethlehem	Canaan	Carroll	Charlestown	Claremont	Enfield	Haverhill	Keene	ancaster

Information obtained from "Public Water Supplies - 1977" published by the New Hampshire Water Supply and Pollution Control Commission.

^{*} Water supply projects considered significant if they regulate a watershed of 1 square mile or greater.

TABLE 4 (continued)
SIGNIFICANT* MUNICIPAL WATER PROJECTS - WATER SUPPLY FACILITIES

COMMUNITY	SOURCE	WATERSHED AREA (mi ²)	DOWNSTREAM BODY OF WATER
Connecticut River Basin (continued)			
Lebanon	Mascoma River	86.0	Mascoma River
Lisbon	Pearl Lake	5.0	Ammonoosuc River
Littleton	Gale River	12.5	Ammonoosuc River
Northumberland	4 Tributaries to Roaring Brook	4.05	Ammonoosuc River
Sunapee	Lake Sunapee	42.6	Sugar River
Stratford	Kimball Pond	5.0	Connecticut River
Whitefield	Ayling Brook	11.7	Connecticut River
Merrimack River Basin			
Andover	Bradley Lake	4.2	Blackwater River
Ashland	Sky & Jackson Ponds	2.0	Pemigewasset River
Boscawen	Walker Pond	4.6	Contoocook River
Concord	Penacook Lake	3.70	Merrimack River
Greenville	Miller & Gamble Brooks	5.6	Souhegan River
Hillsboro	Loon Pond	1.80	Beards Brook
Jaffrey	Bullet Pond	4.0	Contoocook River
Lincoln	Loon Pond	2.35	East Branch Pemigewasset River
Manchester	Lake Massabesic	42.0	Merrimack River

TABLE 4 (continued)

SIGNIFICANT* MUNICIPAL WATER PROJECTS - WATER SUPPLY FACILITIES

COMMUNITY	SOURCE	WATERSHED AREA (mi ²)	DOWNSTREAM BODY OF WATER
Merrimack River Basin (continued)	ntinued)		
Meredith	Lake Waukewan	11.7	Lake Winnipesaukee
Nashua l	Pennichuck Brook & Soughegan River	River not given	Souhegan River
Wilton	Mill & Stockwell Brooks	11.7	Souhegan River
Woodstock	Gordon Pond Brook	6.30	Pemigewasset River
Piscataqua River and New Hampshire Coastal Basin	Hampshire Coastal Basin		
Durham ²	Oyster & Lamprey Rivers	16.9	Oyster & Lamprey Rivers
Exeter	Dearborn River (Exeter River - auxiliary)	1.6	Squamscott River Exeter River
Portsmouth	Bellamy River	20.0	Bellamy River
Rochester	Round Pond	10.25	Cocheco River
Somersworth	Salmon Falls River	not given	Salmon Falls River
Saco River Basin			
Bartlett	Albany River	6.8	Saco River

A proposed change would replace ¹ Flow diverted from Souhegan River to Pennichuck Brook Water Works system. the Souhegan River diversion with flow from the Merrimack River.

² Flow is diverted from the Lamprey River to the Oyster River to supplement the Durham municipal water supply

effluent discharges less than 1.0 cfs (0.65 mgd) were insignificant in comparision to the streamflow occurring in the recipient streams. Water supply projects that regulated a drainage area of 1.0 square mile or more were considered to be significant. However, the influence of water supply projects on low flow values was assumed inconsequential if they regulated less than 5% of the contributing drainage area at the next downstream gaging station. Information pertaining to these municipal water projects was compiled using New Hampshire Water Supply and Pollution Control Commission (NHWSPCC) publications (NHWSPCC, various dates).

The review of all significant municipal water projects (water supply intake and wastewater treatment facilities) revealed that streamflow data of the gages used in the regional analysis were not affected to a significant degree by these projects.

Selection of Variables

Dependent Variables: Low Flow Discharge Rates:

As stated before, one goal of this study was to develop a methodology for estimating natural low flows on streams in ungaged watersheds in New Hampshire. Therefore, the dependent data set consisted of the 7-, 30-, 90-, and 183-day flows at the 2-, 10-, 20-, and 50-year return intervals for the period of record of the 30 gaged watersheds in New Hampshire and bordering areas. Discharge data for the 30 watersheds to be used in the study were obtained from the U.S. Geological Survey (USGS) WATSTORE file (U.S. Department of the Interior, 1980).

Independent Variables: Watershed Characteristics:

The independent variables to be used in this study have been broken into two major groups: climatic and geomorphologic. These variables must reflect interwatershed variations, and must be in a form comparable to the low flow data.

Climate is critical, as it reflects the moisture which comes into the watershed and the allocations of that moisture within the watershed. The first variable selected to represent climate was annual precipitation deviation from the "normal" precipitation (CLAD). This variable was selected to express the "wetness" or "dryness" of an individual year. Since potential evapotranspiration tends to be relatively constant from year to year, this variable (CLAD) should generally reflect the tendency for extreme or modest low flow events in any group of watersheds or within the record of a single watershed.

The second variable selected, estimated mean basin elevation, is a geomorphologic variable closely related to climate. The topography of the state increases in elevation away from the ocean, and the elevation of a watershed expresses aspects of temperature, orographic rainfall, and snowpack. As a result of these climatic factors, less variable flows and relatively higher low flow discharge rates are expected as elevation increases. However, the predominance of bedrock in watersheds at higher elevations tends to minimize storage of water within the catchment and result in greater flow variability. Elevation, therefore, plays a complex role, as it incorporates both climatic and geologic factors.

Within the wide range of potential geomorphologic variables, three others were selected: main channel slope, watershed area, and watershed surface storage. Slopes were thought to reflect general aquifer conditions (i.e., the higher the slope, the more rapid the discharge and the more variable the discharge). Also, steeper sloped areas tend to have more impervious surface materials such as till and bedrock, which speed discharge and create more variable flows.

Watershed area was selected, as the total volume of potential discharge will increase with increasing area. A positive relationship between area and low flow discharges is expected.

Watershed surface storage, as measured by the percentage of watershed area that is occupied by lakes and ponds, was also selected. Increased evaporation losses with increased area of surface storage are expected. In general, however, watershed storage increases should be positively associated with low flows, since storage will augment flows during dry seasons.

The following five variables were selected to statistically model low flow events:

- 1) CLAD
- 2) Elevation
- 3) Main channel slope
- 4) Watershed area
- 5) Watershed surface storage.

Other watershed characeristics were considered for use in this study, but were not included in the final list of independent variables due to their interdependence with the other variables.

lack of available data, or their perceived relative unimportance as influences on low flow discharge rates. For example, data on surficial and subsurface geology, which would indicate aquifer characteristics, are not available for most aquifers in the state. Therefore, a true geologic indicator variable could not be included in the analysis, although mean watershed elevation may be an indirect geologic indicator. Land use, such as the amount of forest cover or impervious area within a watershed, constitutes another category of variables which reflect both the geomorphologic and the climatic processes which impact low flows. However, the amount of data was limited and the variation in these factors across the watersheds in the study area was expected to be small. Climatic factors which were considered but not selected included mean annual precipitation, precipitation intensity (24-hr. raninfall, 2 yr. probability), and July normal temperature. Other investigators (e.g., Lull and Sopper, 1966) have found mean annual precipitation to be more closely related to mean annual runoff than to low flow discharge rates; therefore it was rejected. Precipitation intensity has been included in several investigations of low flows, but the physical basis for its correlation with low flow discharge is unclear. As a variable, July normal temperature would permit comparisons of drought potential between different watersheds, but not comparisons of flows within a single watershed over time. Geographic factors, such as latitude and orientation of the watershed were also considered but their influence on low flows is an indirect one, and should be expressed in part by the elevation

and precipitation variables. Finally, geometric characteristics such as watershed shape and length-width ratios were examined but not included in the final list of variables because their influence on low flow discharge rates was perceived to be minor.

Data Collection

In keeping with the scope of work for this investigation, published reports and studies were the source of much of the data. Most of the watersheds included in this investigation had been studied previously; new data was collected only for those watersheds and for variables which information was lacking.

Average annual low flow rates for a range of durations (1, 3, 7, 15, 30, 60, 90, 120 and 183 days) for the period of record, as well as frequency - discharge values corresponding to these durations were obtained for the 30 watersheds in the study from the USGS WATSTORE data files. All frequencies were computed using a Log-Pearson Type III distribution. These data are contained in Appendix B.

The drainage area in square miles of each watershed was obtained from recent USGS Water Resources Data reports and USGS Water Supply Papers.

Data on estimated mean watershed elevation were obtained from two sources. A recent article by Dingman (1978) contained this information for several watersheds. Using USGS topographic maps for the remainder of the watersheds, estimated mean watershed elevation was calculated according to the following formula developed by Dingman:

$$\overline{E}$$
 = E + 0.324 (E - E) (1) est(i) min(i) max(i) min(i)

where

E = highest elevation in watershed 'i',
max(i) usually located on the drainage divide.

Main channel slopes were obtained from articles by Benson (1962) and LeBlanc (1973) for several watersheds. For the other watersheds, values of main channel slope were calculated according to the following formula, using USGS topographic quandrangle maps:

where
$$S_{(i)} = \frac{(E(85)_{(i)} - E(15)_{(i)})}{D_{(i)}}$$
 (2)

 $S_{(i)}$ = slope for watershed 'i'

E(85)_(i) = the elevation of a point 85% of the distance from the watershed mouth to the endpoint of the main stream in watershed 'i'

 $E(15)_{(i)} =$ the elevation of a point 15% of the distance from the watershed mouth to the endpoint of the main stream in watershed 'i'

 $D_{(i)}$ = the length of the main stream in watershed 'i'

The amount of surface storage area, expressed as a percentage of watershed area, was obtained for some watersheds from Benson (1962). For the other watersheds, it was calculated by planimetering on USGS quadrangle maps (1:24,000 and 1:62,500 scales) the areas of lakes and ponds within a watershed. These areas were converted to a percentage of watershed area, and then 0.5% was added to the value for each watershed to prevent values of zero for this variable. The formula for computing this variable is as follows:

$$St_{(i)} = \frac{(L_{(i)} + P_{(i)})}{B_{(i)}} \times 100 + .005 B_{(i)}$$
 (3)

where

 $St_{(i)}$ = surface storage in watershed 'i'

P(i) = pond storage in watershed 'i' visible at a scale of 1:24000 or 1:62500

B(i) = watershed drainage area.

Data for the climatic variable, CLAD, were collected from National Oceanic and Atmospheric Administration publications (U.S. Dept. of Commerce, 1949-1974, 1964). Twenty-one weather stations with 30 or more years of records were selected across the study area. Thirty years of annual precipitation deviation from normal precipitation data were collected for each weather station. Values of annual precipitation deviation from the normal precipitation (CLAD) were calculated according to the following formula:

$$CLAD(n) = P(n) - \overline{P}$$
 (4)

where

CLAD(n) = annual precipitation deviation from normal
precipitation during year n

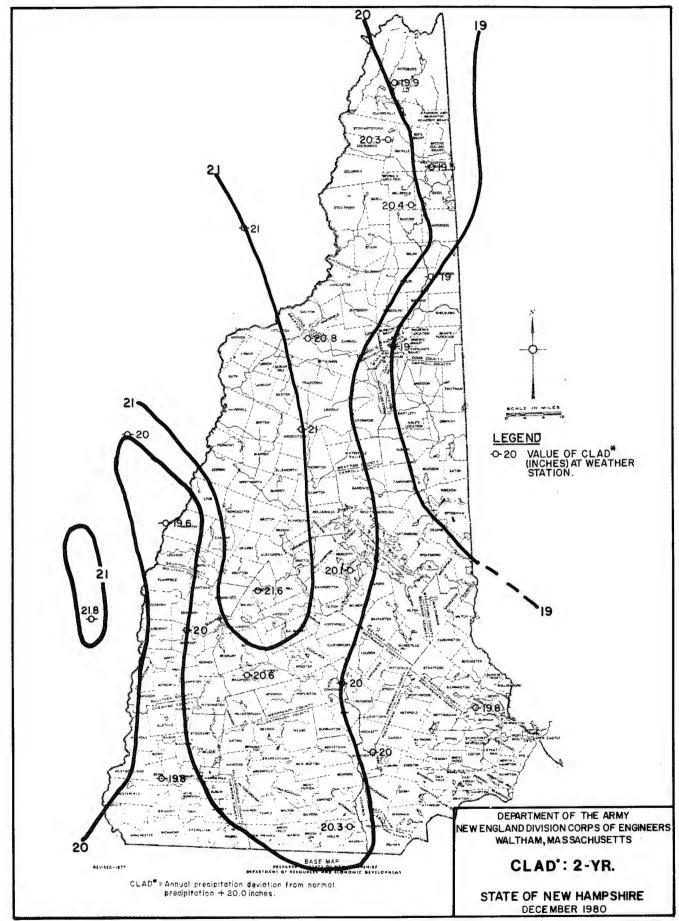
P(n) = annual precipitation for year n

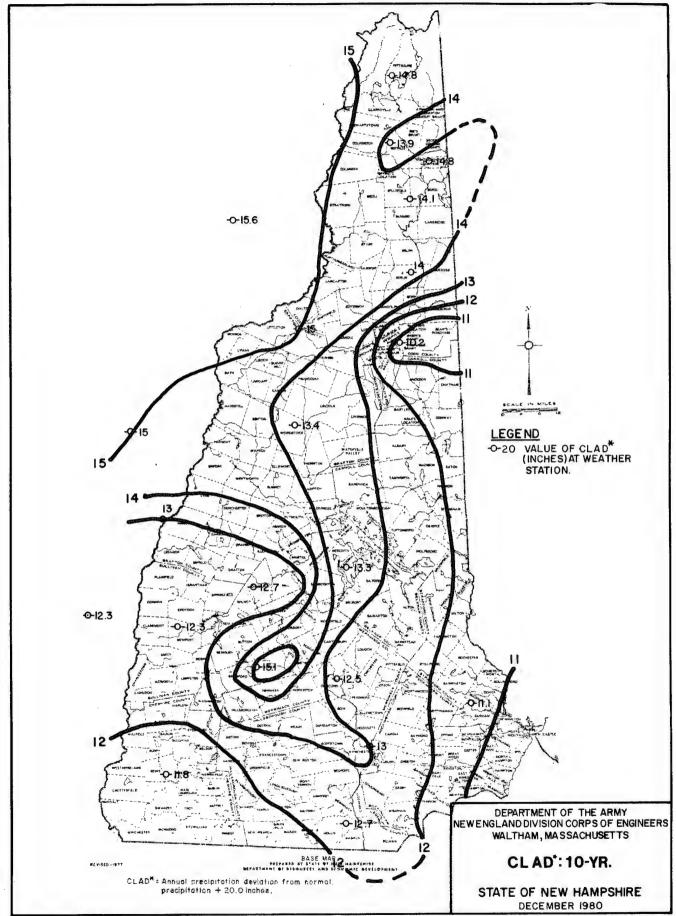
P = climatological normal precipitation based on the period 1941-1970

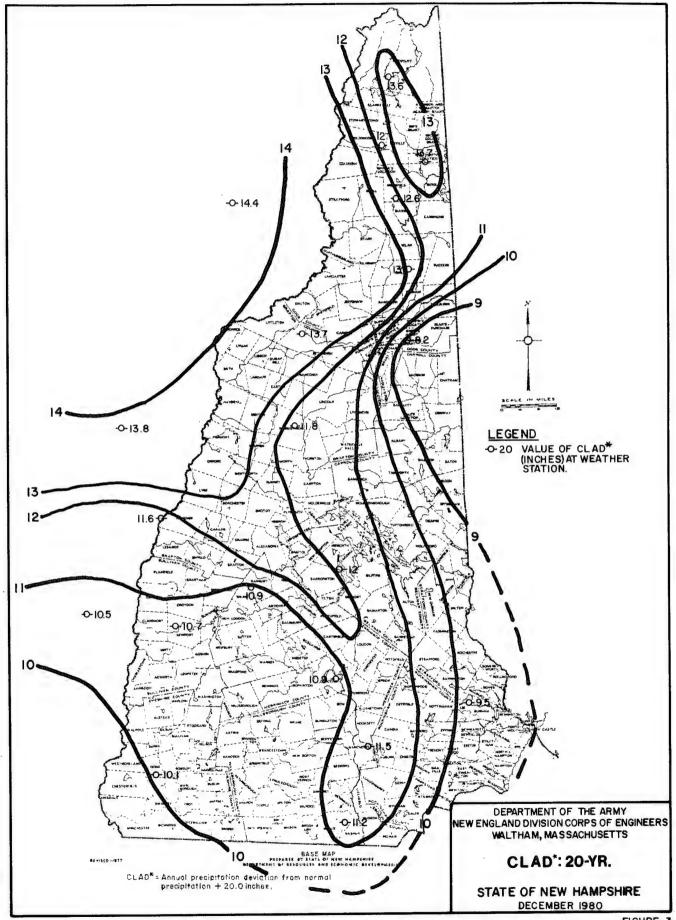
Twenty (20.0) inches were added to each year's value to eliminate negative and/or zero values and allow the numbers to be transformed to logarithms for a later analysis. For each weather station, recurrence intervals for the CLAD + 20.0 (or CLAD*) values were calculated using the USGS plotting position formula (Recurrence Interval = (n+1)/m, where n is the number of years (values) and m is the order number) (Riggs, 1972).

Four maps were constructed, one for each of the four frequencies (2, 10, 20, and 50 years), according to the following procedure. For each weather station, the value of CLAD* for the desired frequency was read off the appropriate graph and plotted on a map. The 21 values (one for each weather station) were then contoured. Input values for the CLAD* variable for the watersheds in the study were interpolated from these contour maps (Figures 1-4).

The streamflow rates, and the morphologic and climatic data comprised the overall data set for the study (see Table 5). The selection of specific discharge or climatic values for the various statistical analyses is outlined in the following section.







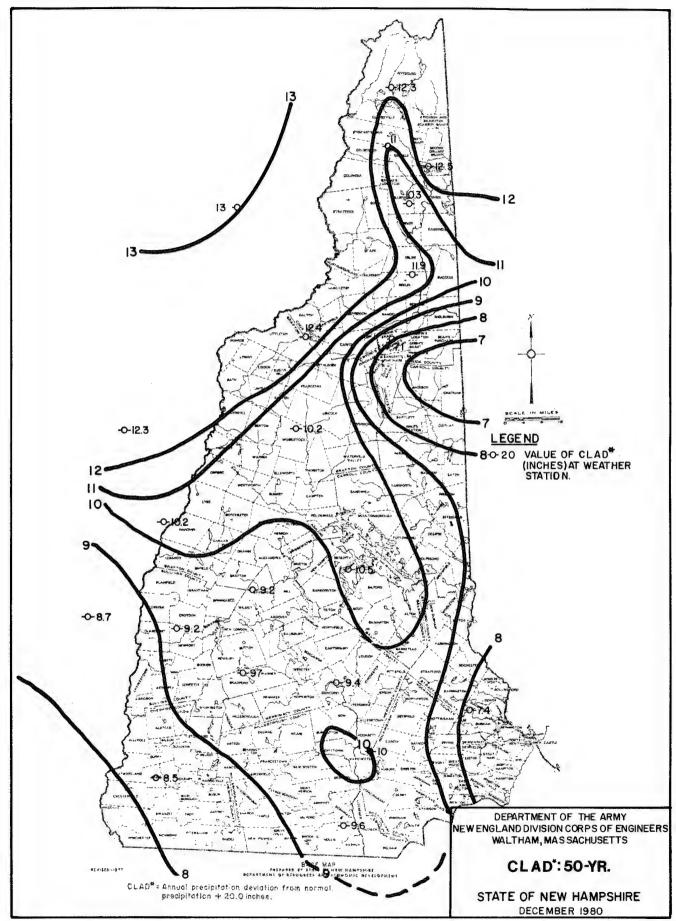


TABLE 5

WATERSHED PARAMETERS FOR GAGES USED IN REGIONAL ANALYSIS

			Drain-				Annua	1 Devis	Deviation + 20.0	20.0	
Map No.	10. No.	Description	Area (Mi ²)	Estimated Mean Basin Elevation	Storage (% DA + 0.5%)	Slope (ft/mi)	2 yr	(CLAD*)	(D*) 20 yr	50 yr	Period of Record
*	052500	Diamond R. nr Wentworth			and other differen	at a see the second of the sec		:			:
		Location, NH	153	2030	1.39	40.14	19.7	13.9	13.2	12.4	1943-1978
2.	054300	Ellis R. at South Andover,									
		Æ	131	1654	2.01	53.77	18.6	13.5	10.5	11.3	1964-1979
m	057000	Little Androscoggin R.									
		nr South Paris, ME	76.2	1053	0.58	51.92	18.6	10.0	8.5	8.0	1915-1979
4.	064300	Ellis R. nr Jackson, NH	10.9	3050	0.50	555.00	19.0	10.2	9	7.5	9761-1961
* 5	064500	Saco R. nr Conway, NH	386	2320	0.83	50.99	8.8	11.0	8	0.7	1905-1978
* 6.	073000	Oyster R. nr Durham, NH	12.1	170	1,82	21.50	19.8	11.5	6.7	0.8	9761-9861
7.	074500	East Branch Pemigewasset R.									
		nr Lincoln, NH	104	2390	0.59	108.80	20.6	13.3	6	10.4	1930-1953
ထ	075000	Pemigewasset R. at									600, 000
		Woods tock, NH	193	2120	0.73	80.70	19.9	13.4	11.9	10.3	1941-1977
<u>ه</u>	000920	Baker R. nr Rumney, NH	143	1890	1.00	107, 10	21.3	14.2	12.7	10.0	1930-1977
10.	076500	Pemigewasset R. nr Plymouth,					•				
		HN	622	2010	0.88	42.00	21.0	13.4	12.0	10.2	1905-1978
*	078000	Smith R. nr Bristol, NH	85.8	1250	1.36	22.60	21.2	13.4	12.1	9.6	1920-1979
12.	084200	Beards Br. nr Hillsboro, NH	55.4	1214	2.41	78.30	20.6	14.0	10.6	9.5	1947-1970
3	086000	Warner R. nr Davisville, NH	146	1150	2.51	31.80	20.8	14.9	10.8	9.5	1941-1978
14.	082000	Blackwater R. nr Webster, NH	129	1240	2.09	24.50	21.0	14.0	10.9	15	1920-1979
*15.	083000	Soucook R. nr Concord, NH	76.8	680	0.69	33.20	20.0	13.0	12.0	9.6	1953-1980
*16.	091000	South Branch Piscataquog R.									
		nr Goffstown, NH	104	889	1.16	30.60	20.3	13.1	10.5	8.6	1942-1978
17.	000460	Souhegan R. at Merrimack, NH	171	850	. 0.87	31,20	20.3	12.5	10.6	6	1911-1976
18.	130000	Upper Ammonoosuc R. nr									
		Groveton, NH	232	1970	1.22	28.60	20.4	14.3	13.3	12.3	1942-1980
*19.	133000	East Branch Passumpsic, R.									
		nr East Haven, VT	53.8	1666	1.19	60.50	20.9	15.6	14.4	12.9	1941-1929
20.	134500	Moose R. nr Victory, VT	75.2	1865	1.19	78.40	20.9	15.4	14.1	12.6	1948-1979
	;	: :									

* Watersheds used in model testing

TABLE \$ (Continued)
WATERSHED PARAMETERS FOR GAGES USED IN REGIONAL ANALYSIS

	Period of Breeze	1930-1979	1941-1979 1937-1979 1941-1978	1941-1978	1942-1979	1942-1979 1942-1978 1930-1961	1922-1978
20.0	50 45	12.5	12.0	10.1	8.6	8.9.8	8.6
Annual Deviation + 20.0 (CLAD*)	2 yr 10 yr 20 vr 50 yr	14.1	13.0	12.1	10.4	0.00 8.05 8.05	10.2
l Devia (CLA	10 vr	15.4	14.0	13.3	12.2	12.2	11.8
Аппиа	2 yr	.21.0	20.5 21.2 20.1	21.0	20.9	20.5 19.7 20.7	19.9
i	(ft/mi)	40.40	72.00 28.70 80.4	50.2	56.50	87.40 49.00 31.7	100.00
Storage	+ 0.5%)	0.75	0.51	2.36	0.50	0.56	1.97
Estimated Mean Basin	Elevation	1500	2840 2340 1165	1610	1300	1171 960 1561	1480
Drain- age Area	(M12)	128	87.6 395 30.5	80.5	103	77.2 82.7 308	36.0
	Description	Moose R. nr St. Johnsbury, VT Ammonoosuc R. nr Bethlehem	Jct., NH Ammonoosuc R. nr Bath, NH Ayers Br. at Randolph, VT Mascoma R. at West Canaan,	Williams R. at Brockaway	Saxtons R. at Saxtons River,	Cold R. at Drewsville, NH West R. at Newfane, VT South Branch Ashuelot R. nr	mariborough, NH
USGS Map ID. No.	-10	135000	138000 142500 145000	153500	154000	155000 156000 160000	
Map	. O.	*22.	.23. *24. 25.	*26.	27.	28. *29. 30.	

* Watersheds used in model testing

Application of the Modeling Methodology

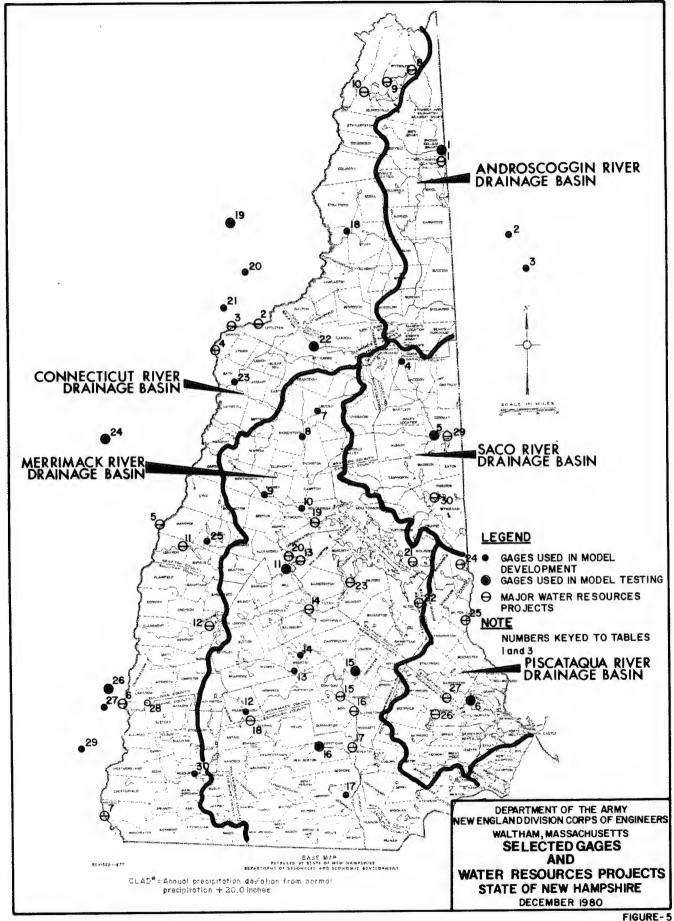
One stated objective of the project was to predict low flows for the following periods: 7 days, 30 days, 90 days, and 183 days. This was accomplished by relating the previously discussed five variables to the measured low flow data.

The methodology used was the development of a model using regression analysis to predict each of the following:

- a) the 2-, 10-, 20- and 50-year seven-day low flows
- b) the 2-, 10-, 20- and 50-year thirty-day low flows
- c) the 2-, 10-, 20- and 50-year ninety-day low flows
- d) the 2-, 10-, 20- and 50-year one hundred and eighty-three-day low flows.

In applying regression, it is good practice to have a sample for model derivation, as well as a sample for model testing. The test sample helps to insure that the derived model does not incorporate major predictive errors. Therefore, the sample of 30 watersheds was stratified into two groups. The first group consisted of 19 watersheds and was used for model derivation, while the second group of 11 watersheds was used to test the derived models. Figure 5 shows the location of the 30 gages as well as the major water resources projects within the state.

For the model derivation group of 19 watersheds, values of the 7-, 30-, 90-, and 183-day low flows at each of the four return intervals being investigated were selected. The value of CLAD* for each return interval was read from the appropriate map. The associated geomorphologic data were also collected for each



watershed. The five climatic and geomorphologic variables were assumed to be normally distributed and independent, and the relationships between the selected variables were assumed to be linear. All flows were converted to cubic feet per second per square mile (csm) to control for the overwhelming influence of watershed area on flow magnitude. The variables were related to the low flows through the use of a forward stepwise multiple linear regression program of the SPSS (Statistical Package for the Social Sciences, 1970). The resultant equations were:

$$Q_{7csm} = 0.0001E + 0.0086 CLAD* - 0.0288 St + 0.0002S + 0.0001 AREA - 0.1395$$
 (5)

$$Q_{30csm} = 0.0001E + 0.0130 CLAD* - 0.0364 St + 0.0002S + 0.0001 AREA - 0.1846$$
 (6)

$$Q_{90csm} = 0.0002E + 0.0239 CLAD* -0.0539 St + 0.0003S + 0.00004 AREA - 0.2969$$
 (7)

$$Q_{183csm} = 0.0002E + 0.0489 CLAD* - 0.0870 St + 0.0008S + 0.00003 AREA - 0.5308 (8)$$

where Q_{ncsm} = discharge in cubic feet per second per square mile for the duration of n days

E = estimated mean basin elevation,

CLAD* = annual precipitation deviation from normal
 precipitation for the recurrence interval being
 investigated + 20.0

S = main channel slope,

St = basin storage, and

AREA = drainage area.

It is interesting to note that elevation and CLAD* explain most of the variance in the data (see Table 6). These variables account for the following percentage explanation of variance of low flows:

Q7csm: CLAD* and E explain 66% of the variation Q30csm: CLAD* and E explain 71% of the variation CLAD* and E explain 75% of the variation Q183csm: CLAD* and E explain 75% of the variation

The general increase in variance explanation with the increasing period of low flow is probably due to the increased sensitivity of the climatic variable to long-term droughts.

Since the variance explanation of these two variables is consistently high and accounts for virtually all of the variance explanation, linear regression equations were developed using these two variables alone:

$$Q_{7csm} = 0.00014E + 0.00780 CLAD* - 0.20959$$
 (9)

$$Q_{30csm} = 0.00016E + 0.01209 CLAD* - 0.27042$$
 (10)

$$Q_{90csm} = 0.00023E + 0.02231 CLAD* - 0.42554$$
 (11)

$$Q_{183csm} = 0.00037E + 0.04496 CLAD* - 0.75804$$
 (12)

The F-test was used to assess whether the inclusion of successive independent variables in the regression significantly improved the amount of variation in the dependent variable that is explained by multiple regression (Till, 1974, p.140). The results of the F-tests (see Table 6) indicate that each of the regressions is significant at the 0.05 level of confidence.

TABLE 6

Variance Explanation and Significance of Regression Equations

Classifications Classifica	Flow Duration	Variable	Variance Explanation (R ²) (Coefficient of Determination).	F Value	Degrees of Freedom m	Standard Error
ELEV. CLAD*. STOR. SLOPE. ELEV. CLAD*. STOR. SLOPE ELEV. CLAD*. SLOPE ELE	Linear Equations					-
ELEV, CLAD# STOR CLAD# CLAD# STOR CLAD# STOR CLAD# CLAD# STOR CLAD# CLAD# STOR CLAD# STO	0,	ELEV	.57	97	1 74	.00
ELEV. CLAD*, STOR, SLOPE, AREA		E CLAD*	99.	72	2 73	90.
ELEV. CLAD*, STOR, SLOPE, AREA		CLAD*, STOR	. 70	22	3 72	90.
ELEV CLAD*, STOR, SLOPE, AREA 72 3.6 5 70 ELEV CLAD*, STOR, SLOPE, AREA 75 ELEV, CLAD*, STOR, SLOPE, AREA 76 ELEV, CLAD*, STOR, SLOPE, AREA 76 ELEV, CLAD*, STOR, SLOPE, AREA 81 ELEV, CLAD*, SLOPE, STOR 75 ELEV, CLAD*, SLOPE, STOR 76 ELEV, CLAD*, SLOPE, STOR 77 ELEV, CLAD*, SLOPE, STOR 76 ELEV, CLAD*, SLOPE, STOR 77 ELEV,		CLAU*, SIOR, SLOPE	. 17.	44	17 4	90.
ELEV, CLAD*, STOR ELEV, CLAD*, STOR ELEV, CLAD*, STOR ELEV, CLAD*, STOR, SLOPE ELEV, CLAD*, SLOPE, STOR, AREA ELEV, CLAD*, STOR, STOR,		CLAD", SIUR, SLOPE,	.72	36	5 70	90.
ELEV, CLAD*, STOR ELEV, CLAD*, STOR, SLOPE, AREA ELEV, CLAD*, STOR, SLOPE, STOR ELEV, CLAD*, STOR, SLOPE, STOR ELEV, CLAD*, SLOPE, STOR, AREA ELEV, CLAD*, SLOPE, STOR, S	030		.55	16	1 . 74	60.
ELEV. CLAD*, STOR, SLOPE ELEV. CLAD*, SLOPE ELEV. CLAD* ELEV. CLA	3	CLAD*		89	2 73	.07
ELEV, CLAD*, STOR, SLOPE 76 55 4 71 ELEV, CLAD*, STOR, SLOPE, AREA 76 55 4 71 ELEV, CLAD*, STOR, SLOPE AREA .75 111 2 73 4 71 ELEV, CLAD*, STOR, SLOPE AREA .80 73 4 71 72 ELEV, CLAD*, STOR, SLOPE AREA .43 55 1 74 76 ELEV, CLAD*, SLOPE STOR .43 55 1 74 71 ELEV, CLAD*, SLOPE STOR .81 78 4 71 ELEV, CLAD*, SLOPE STOR .81 61 5 70 ELEV, CLAD*, SLOPE STOR .46 64 1 74 ELEV, CLAD*, SLOPE STOR .46 64 1 74 ELEV, CLAD* .46 .46 .77 .47 .74 ELEV, CLAD* .46 .46 .74 .74 .74 ELEV, CLAD* .46 .47 <td></td> <td>CLAD*, STOR</td> <td>. 75</td> <td>72</td> <td>3 72</td> <td>. 07</td>		CLAD*, STOR	. 75	72	3 72	. 07
ELEV, CLAD*, STOR, SLOPE, AREA . 76 44 5 70 ELEV, CLAD*, STOR ELEV, CLAD*, SLOPE ELEV, CLAD*, STOR ELEV, CLAD*,	•	CLAD*, STOR, SLOPE	.76	55	4 71	90.
ELEV CLAD* .50 75 111 2 73 ELEV, CLAD* .77 .79 .90 .3 .72 ELEV, CLAD* .510R. .81 .83 .73 .41 .71 ELEV, CLAD* .510R. .43 .58 .5 .70 .70 ELEV, CLAD* .510PE .81 .73 .72 .73 .72 ELEV, CLAD* .510PE .81 .71 .74 .71 .71 ELEV, CLAD* .510PE .81 .71 .74 .71 .74 .71 ELEV, CLAD* .510PE .77 .74 <td></td> <td>CLAD*, STOR, SLOPE,</td> <td>. 76</td> <td>44</td> <td>5 · 70</td> <td>90°</td>		CLAD*, STOR, SLOPE,	. 76	44	5 · 70	90°
ELEV. CLAD*, STOR CLAD*, STOR CLAD*, STOR CLAD*, STOR SLOPE	0.00		.50	75	1 74	.13
ELEV, CLAD*, STOR, SLOPE ELEV, CLAD*, STOR, SLOPE ELEV, CLAD*, STOR, SLOPE ELEV, CLAD*, STOR, SLOPE ELEV, CLAD*, SLOPE ELEV, CLAD* ELEV E		CLAD*	.75		2 73	.09
CLIV', CLAD*, STOR, SLOPE, AREA .81 .83 .4 .71		CLAD*, STOR	. 79	90	3 72	60.
ELEV, CLAD*, STOR, SLORE, AREA .81 58 5 70 ELEV, CLADA*, SLOPE, STOR ELEV, CLADA*, SLOPE, STOR, AREA .81 ELEV, CLADA* ELEV, C		CLAD*, STOR, SLOPE	08.	73	14 71	80.
ELEV CLAD* CLAD*, SLOPE, STOR ELEV, CLAD*, SLOPE, STOR, AREA ELEV, CLAD* ELEV,		CLAD*, STOR, SLORE,	.81	58	5 70	.08
ELEV, CLAD*, CLAD*, SLOPE, SLOPE, STOR ELEV, CLAD*, SLOPE, STOR ELEV, CLAD*, SLOPE, STOR ELEV, CLAD*, SLOPE, STOR STO	0,83		.43	55	1 74	. 26
ELEV, CLAD*, SLOPE, STOR, AREA81 78 44 771 ELEV, CLAD*, SLOPE, STOR, AREA81 61 55 70 ELEV, CLAD*, CLAD* ELEV, CLAD* CLAD* CLAD* CLAD* 11 74 102 2 73 CLAD* CLAD* 115 2 73		CLAD*	.75	107	2 73	17
ELEV, CLAD*, SLOPE, STOR, AREA .81 78 4 71 ELEV, CLAD*, SLOPE, STOR, AREA .81 5 70 ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* CLAD* CLAD* CLAD* CLAD* 1 74 74 74 74 74 74 74 74 74 74		CLAD*, SLOPT.	6/.	88	3 72	91.
ELEV, CLAD*, SLUFE, SIUK, AKEA ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* ELEV, CLAD* CLAD*, ELEV CLAD*, ELEV The clade of		CLAD*, SLOPE, STOR	.	78	11/	.15
ELEV GLAD* 46 64 73 73 73 73 74 75 75 73 74 75 75 74 75 74 74 74 74 74 74 74 74 74 74 74 74 75 75 74 74 75 .		CLAD*, SLOPE, STOR,	- 80.	19	5 70	.15
ELEV . CLAD* . 46 64 1 74 74 75 75 73 2 73 74 74 74 74 74 74 74 74 75 74 74 74 74 75 75 75 75 75 75 75 75 75 75 75 75 75	og Equations					
FILEV. CLAD* 5.7 5.7 5.7 7.3 7.4 5.7 5.7 5.7 5.3 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4 5.4	4,	ELEV	94.	49	1 74	.28
ELEV, CLAD* .49 .72 1 74 .74 ELEV, CLAD* .48 .67 74 73 .74 .74 .74 .74 .74 .74 .74 .74 .74 .74			64.	52	2 . 73	.24
ELEV, CLAD* 67 74 . 2 . 73 ELEV, CLAD* 48 . 67 . 1 . 74 CLAD* . ELEV 42 . 53 . 1 . 74 CLAD*, ELEV 76 115 . 2 . 73	030		64.	72	1 74	.24
ELEV, CLAD* 1 74 74 102 2 73 1 74 CLAD*, ELEV 53 1 74 73 75 115 2 73	3		. 67	74	2 73	.20
CLAD* 102 2 73 74 CLAD* 102 2 73 74 CLAD*, ELEV 75 75 75 75 75 75 75 75 75 75 75 75 75	040		84.	29	1 74	23
CLAD*, ELEV .42 53 1 74 CLAD*, ELEV .76 115 2 73			174	102	2 73	.17
.76 115 2 73	9183	CLAD*	.42	53	1 74	.22
		CLAD*, ELEV	. 76	115	2 73	71.

IABLE 6 (Continued)

Variance Explanation and Significance of Regression Equations

Flow Duration	Variable	Variance Explanation (R ²)	F Value	Degrees o	Degrees of Freedom	Standard Fron
Inal Equations		(conficient of Determination)		8	c	
47			161	-	7.7	90°.
030		. 11.	246	-	. 7.	90.
060			297	·	74	80.
0,183		. 78	258	-	74	91.

The predictive ability of the equations was assessed through the use of a 't' differences test (Till, 1974). This tests the hypothesis that the mean difference between predicted and measured low flows is equal to zero, and takes the form

$$t = (\overline{d} - 0) / (S_{\overline{x}})$$

where

t = 't' value

d = the mean difference between predicted and measured low flows

Sx = s/17

s = the standard deviation of the d values

n = sample size of the d values

All of the computed 't' values fell below the critical tabled values at the 0.01 level, allowing acceptance of the hypothesis (H_0) that the mean difference between predicted and measured low flows is not significantly different from 0. The correlations and results of the 't' test suggest that the model should be capable, in general, of predicting low flows.

Before the models were accepted as the final results of the study, they were applied to the test sample of 11 basins. If the correlations and 't' values were similar, then the models could be considered capable of predicting flows in ungaged areas. The results of the analysis were disappointing:

Flow	Correlation	t value	Result
Q ₇	0.77	2.54	accept H _o
Q ₃₀	0.77	3.54	reject H _o
Q ₉₀	0.82	2.96	reject H _o
Q ₁₈₃	0.89	3.13	reject H _o

Although the correlations were similar, the mean differences (between what was predicted and what was measured) were significantly greater than 0 for all but the 7-day low flows. With these results, a model of this nature cannot be considered reliable for application in ungaged basins.

For simplicity, one of the critical assumptions made in the analysis was that variable interrelationships with flow were linear in nature. This may not be the case, so scatter plots of the relationships between each variable and low flow were compiled. These plots indicated that curvilinear relationships did exist between the independent variables and low flows. Since this might improve the predictive power of the models, a curvilinear approach was then used.

All of the data were reviewed to insure that no negative or zero values existed. The climatic, elevation, and flow data were then transformed into base ten logarithms, and subjected to a second multiple linear regression analysis. As before, stepwise multiple regression was performed, in which the variables are entered into the equation in order of decreasing contribution to variance explanation. The resultant models were:

$$\log Q_{7csm} = 1.65 \log E + 1.05 \log CLAD* - 7.48$$
 (14)

$$\log Q_{30csm} = 1.52 \log E + 1.10 \log CLAD* - 6.97$$
 (15)

$$Log Q_{90csm} = 1.39 Log E + 1.28 Log CLAD* - 6.56$$
 (16)

$$Log Q_{183csm} = 1.16 Log E + 1.33 Log CLAD* - 5.62$$
 (17)

These curves were then transformed back into standard form:

$$Q_{7csm} = (E^{1.65}) (CLAD*^{1.05}) / 10^{7.48}$$
 (18)

$$Q_{30csm} = (E^{1.52}) (CLAD*^{1.10}) / 10^{6.97}$$
 (19)

$$Q_{90csm} = (E^{1.39}) (CLAD*^{1.28}) / 10^{6.56}$$
 (20)

$$Q_{183csm} = (E^{1.16}) (CLAD*^{1.33}) / 10^{5.62}$$
 (21)

The correlations were slightly lower than those of the linear models, but the regressions were significant at the 0.05 level (see Table 6). Application of the 't' test to each model indicated that the differences were not significantly different from 0 at the 0.01 level. These models were then applied to the test sample of 11 watersheds, creating the following results:

Flow	Correlation	t value	Result
Q_7	0.80	2.97	reject H _o
Q ₃₀	0.79	3.01	reject H _o
Q ₉₀	0.83	3.08	reject H _o
Q ₁₈₃	0.91	4.31	reject Ho

Again, the model failed to predict values that exhibit little difference with measured values.

A review of the sources of major differences within the 19 basin sample revealed an interesting characteristic. Error increased with the size of low flow. Since the regression technique used to derive these models is based on logarithms, the curve must pass through the origin of the system of data values. If a second regression is applied to equations (18), (19), (20), and (21), the curve orientation and intercept values can be adjusted to fit the data set more effectively.

Since we are dealing with a bivariate situation (both CLAD* and E are combined as one variable), simple regression was used. The resultant equations were:

$$Q_{7csm} = 1.23 \left[(E^{1.65})(CLAD*1.05) / 10^{7.48} \right] - 0.0123$$
 (22)

$$Q_{30csm} = 1.16 \left[(E^{1.52})(CLAD*1.10) / 106.97 \right] - 0.0134$$
 (23)

$$Q_{90csm} = 1.10 \left[(E^{1.39})(CLAD^{*1.28}) / 106.56 \right] - 0.0115$$
 (24)

$$Q_{183csm} = 1.17 \left[(E^{1.16})(CLAD^{*1.33}) / 10^{5.62} \right] - 0.0361$$
 (25)

All of the regressions were significant at the 0.05 level, and the correlations show a marked improvement over the unadjusted curves (see Table 6). In all but one case (\mathbb{Q}_{183}), the correlations are equal to or better than those of the original equations.

Models (22), (23), (24), and (25) were also subjected to 't' testing, and the mean differences were not significantly different from 0 for all four models. The above models were then applied to the test sample of 11 basins, with successful results:

Flow	Correlation	t value	Result
Q ₇	0.80	2.03	accept H _o
Q ₃₀	0.79	2.45	accept H _o
Q ₉₀	0.83	2.51	accept H _o
Q ₁₈₃	0.91	2.69	accept H _o

These results indicate that the derived models are capable of predicting flows in ungaged watersheds, will explain up to 80% of the flow variance, and should be able to predict low flows of intermediate recurrence intervals (2 years < Flow < 50 years).

DISCUSSION OF RESULTS

The technique used here has provided a reasonable method for low flow estimation. The test sample of 11 watersheds is scattered all over the state and adjacent areas of eastern Vermont, and represents the range of conditions over which this model will apply. Used within the limits of the sample (watershed size and geographic limits of the sample area), this model should be an effective tool for estimating low flow discharges. It should be noted that because the final equations do not pass through the origin, the derived models may give negative flow estimates for streams in watersheds at

low elevations (mean watershed elevation less than 600 feet). In the case of negative flow estimates, a minimum estimate of .0001 csm could be used.

There are several areas for potential model improvement. The first area deals with data inputs. Because of the limited number of watersheds sampled for flow, and the limited physiographic data base, a number of more productive techniques had to be eliminated from the study. As the data set improves, this should improve predictive potentials in this area.

A second problem is involved with the statistical procedures used to derive the final models. There are potential issues involved with low flow data serial correlations that could not be considered due to the limited amounts of data. As such, these potential issues were assumed to be "not significant". As the data set improves, there will be an opportunity to research this further.

The regression techniques used here are not a standard polynomial or harmonic approach, and may violate certain assumptions of input data normality. The consistent predictive power of the models exhibited in the analysis of test watersheds is judged to be indicative that no significant error is introduced by these procedures.

Finally, while the derived curves do not purport to represent an optimum fit to the data, "optimizing procedures" were utilized which have produced a curve that approaches optimum fit to the data. A further detailed statistical analysis of residuals would be required

to significantly improve curve form. However, it is judged that the analysis would not significantly improve the predictive powers of the models.

IV. CONCLUSIONS

- 1. Four equations (one each for the 7-, 30-, 90- and 183-day durations) have been derived which are capable of estimating flows in small to intermediate sized watersheds (10-600 square miles). As noted previously, caution must be used when applying these models to low-lying (mean elevation less that 600 ft) watersheds.
- Each model is capable of estimating along a continuum of events, suggesting that the selected variables are strongly associated with interwatershed fluctuations in low flows.
- These equations are valid within the state of New Hampshire.
 Extreme caution must be used when applying these models outside the limits of New Hampshire, as variable interrelationships may change.

V. APPLICATION OF THE MODEL

The equations developed in this study can be used to estimate flows of 7-, 30-, 90- and 183-day durations, in intermediate sized (10 to 600 square miles) watersheds in New Hampshire. The equations are:

$$Q_{7csm} = 1.23 \left[(E^{1.65}) (CLAD^{*1.05}) / 10^{7.48} - 0.0123 \right]$$
 $Q_{30csm} = 1.16 \left[(E^{1.63}) (CLAD^{*1.10}) / 10^{6.97} - 0.0134 \right]$
 $Q_{90csm} = 1.10 \left[(E^{1.39}) (CLAD^{*1.28}) / 10^{6.56} - 0.0115 \right]$
 $Q_{183csm} = 1.17 \left[(E^{1.16}) (CLAD^{*1.33}) / 10^{5.62} - 0.0361 \right]$

With maps of contoured values of the climatic variable, CLAD*, which are provided in Section III of this report, flows can be estimated for four recurrence intervals: 2, 10, 20, and 50 years. The procedure for using these maps and the above models is as follows:

- Determine size of the drainage area in square miles by measuring from USGS topographic maps.
- 2) Select the equation for the flow duration period (7, 30, 90, 183 days) for which flow is to be estimated.
- 3) Select the appropriate CLAD* map for the recurrence interval (2, 10, 20, 50 years) which is being considered.

- 4) Locate the watershed being investigated on the CLAD* map and interpolate a value for the CLAD* variable in the equation that represents the mean CLAD* for the watershed.
- 5) Using USGS topographic maps, determine the estimated mean elevation (E) of the watershed being investigated using Equation (1) in this report (p. 39).
- 6a) Enter the values for CLAD* and estimated mean watershed elevation (E) in the equation and calculate the estimated discharge in cubic feet per second per square mile (csm) for the selected flow duration and return interval.
- OR 6b) Select the Low Flow Guide Curve for the flow duration for which flow is to be estimated (see Figures 6-9). Enter the graph with the estimated mean watershed elevation and read across to the curve which represents the value for CLAD* for the watershed. Then read down to determine the estimated discharge in cubic feet per second per square mile (csm).
 - 7) Multiply by drainage area to determine discharge.

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APPENDIX A

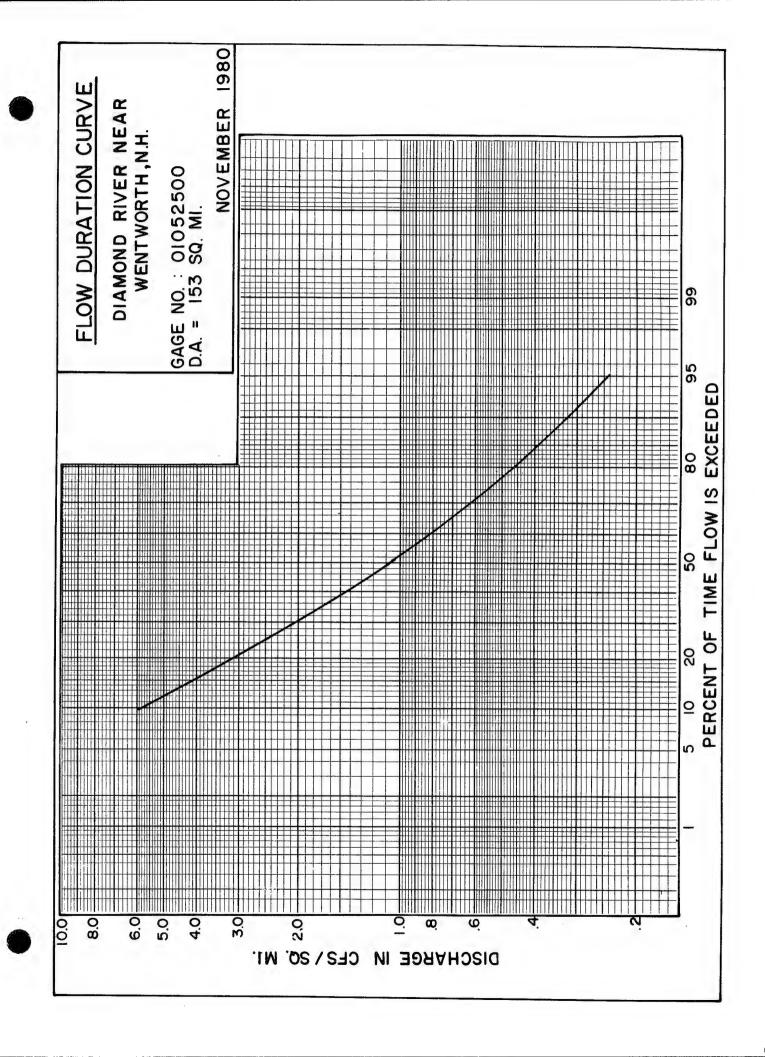
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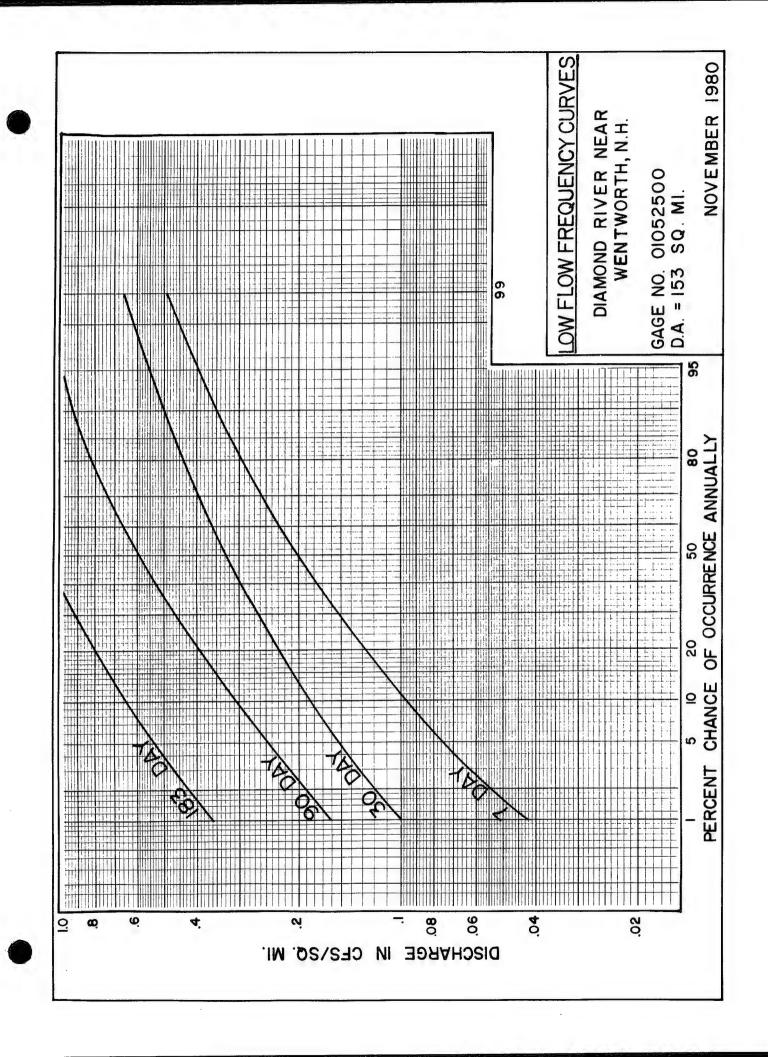
LOW FLOW FREQUENCY CURVES

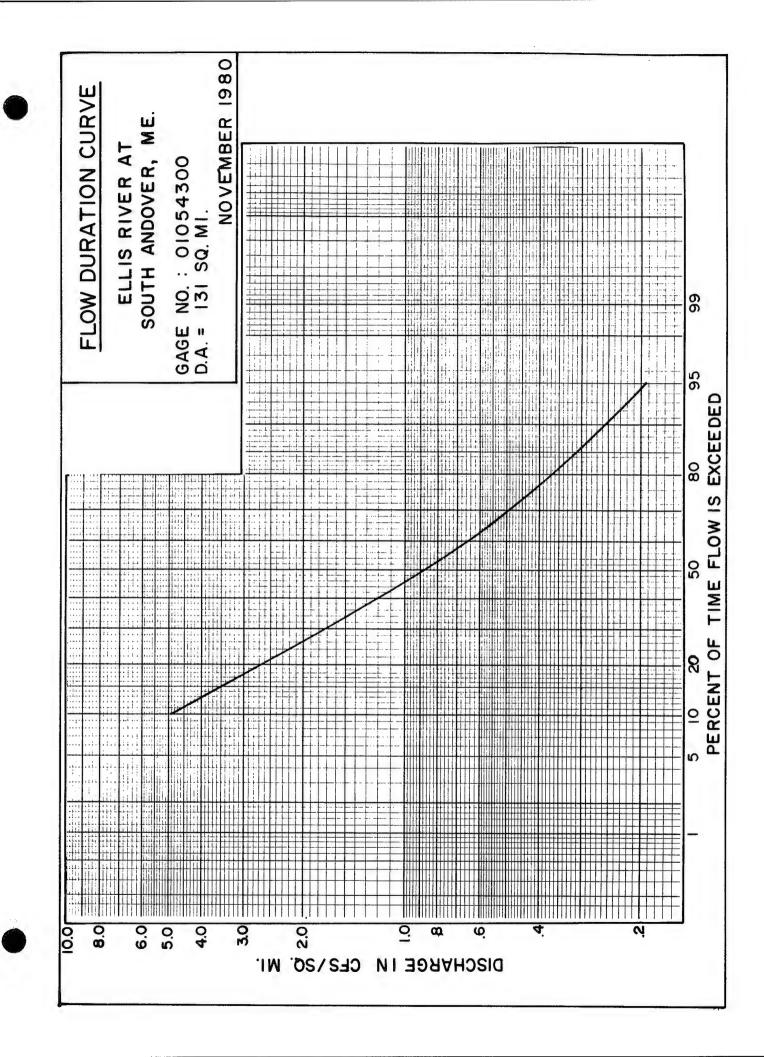
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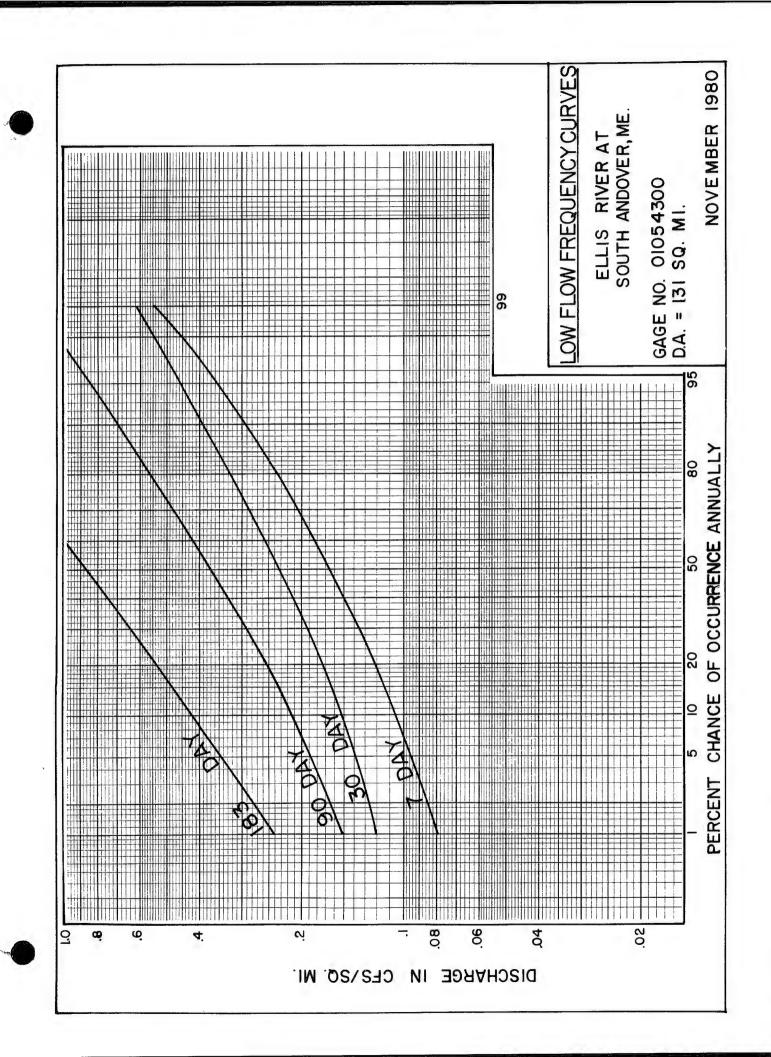
Station No.	Name
010 52500	Diamond River near Wentworth, NH
01054300	Ellis River at South Andover, ME
01057000	Little Androscoggin River near South Paris, ME
01064300	Ellis River near Jackson, NH
01064500	Saco River near Conway, NH
01073000	Oyster River near Durham, NH
01074500	E. Branch Pemigewasset River near Lincoln, NH
01075000	Pemigewasset River at Woodstock, NH
01076000	Baker River near Rumney, NH
01076500	Pemigewasset River near Plymouth, NH
01078000	Smith River near Bristol, NH
01084500	Beards Brook near Hillsboro, NH
01086000	Warner River near Davisville, NH
01087000	Blackwater River near Webster, NH
01089000	Soucook River near Concord, NH
01091000	S. Branch Piscataquoq River near Goffstown, NH
01094000	Souhegan River at Merrimack, NH
01130000	. Upper Ammonoosuc River near Groveton, NH
01133000	E. Branch Passumpsic River near East Haven, VT
01134500	Moose River near Victory, VT
01135000	Moose River near St. Johnsbury, VT

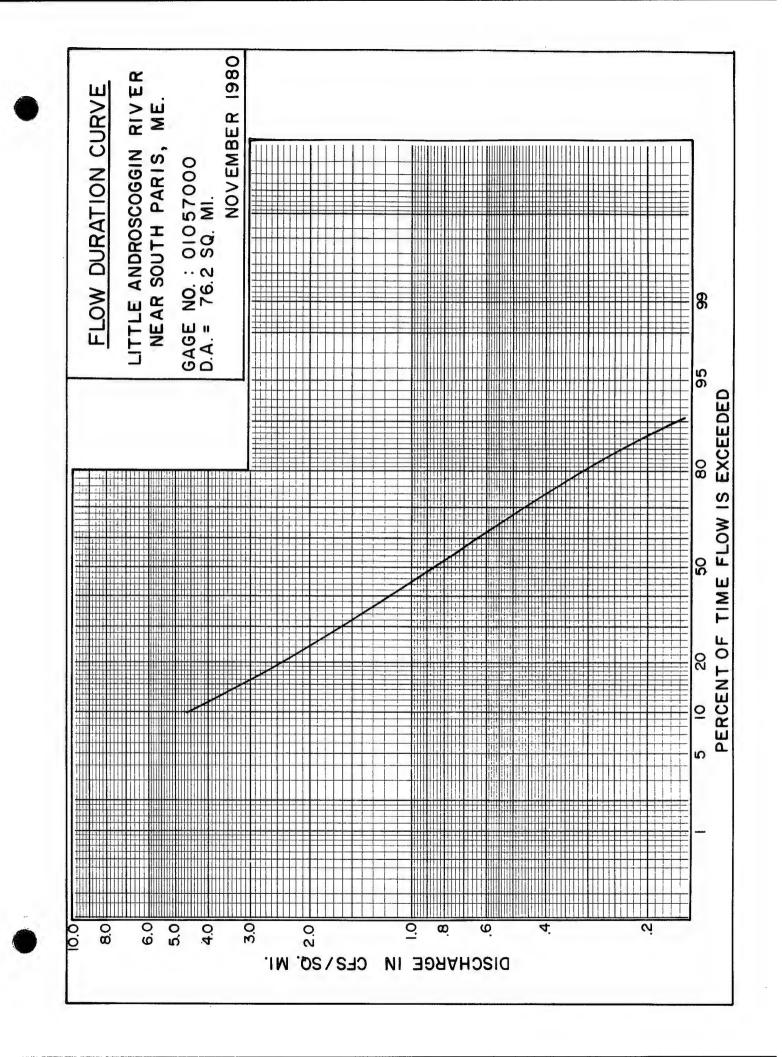
Station No.	Name
01137500	Ammonoosuc River near Bethlehem Jct., NH
01138000	Ammonoosuc River near Bath, NH
01142500	Ayers Brook at Randolph, VT
01145000	Mascoma River at West Canaan, NH
01153500	Williams River at Brockaway Mills, VT
01154000	Saxtons River at Saxtons River, VT
01155000	Cold River at Drewsville, NH
01156000	West River at Newfane, VT
01160000	S. Branch Ashuelot River near Marlborough, NH

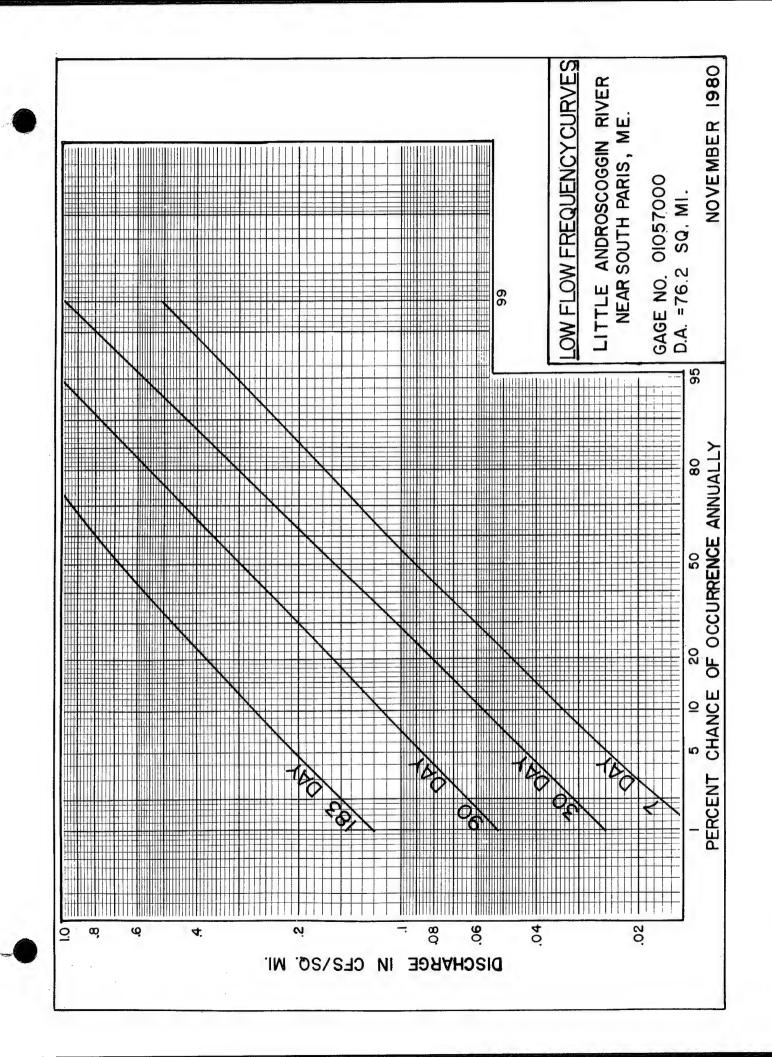


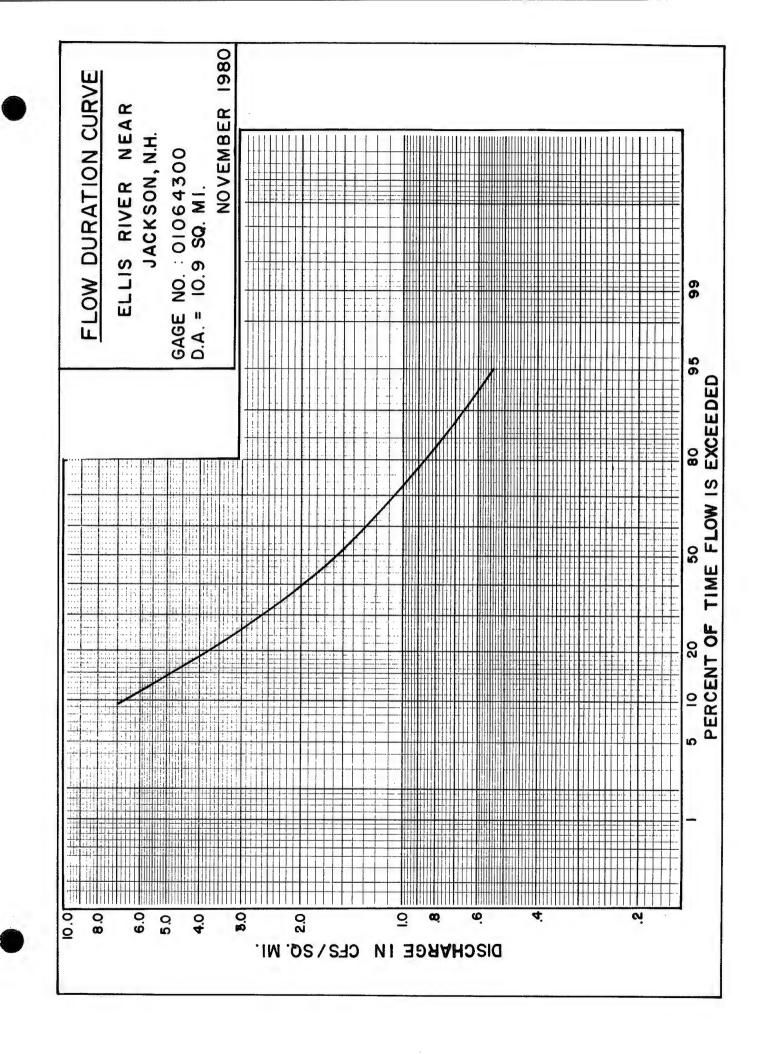


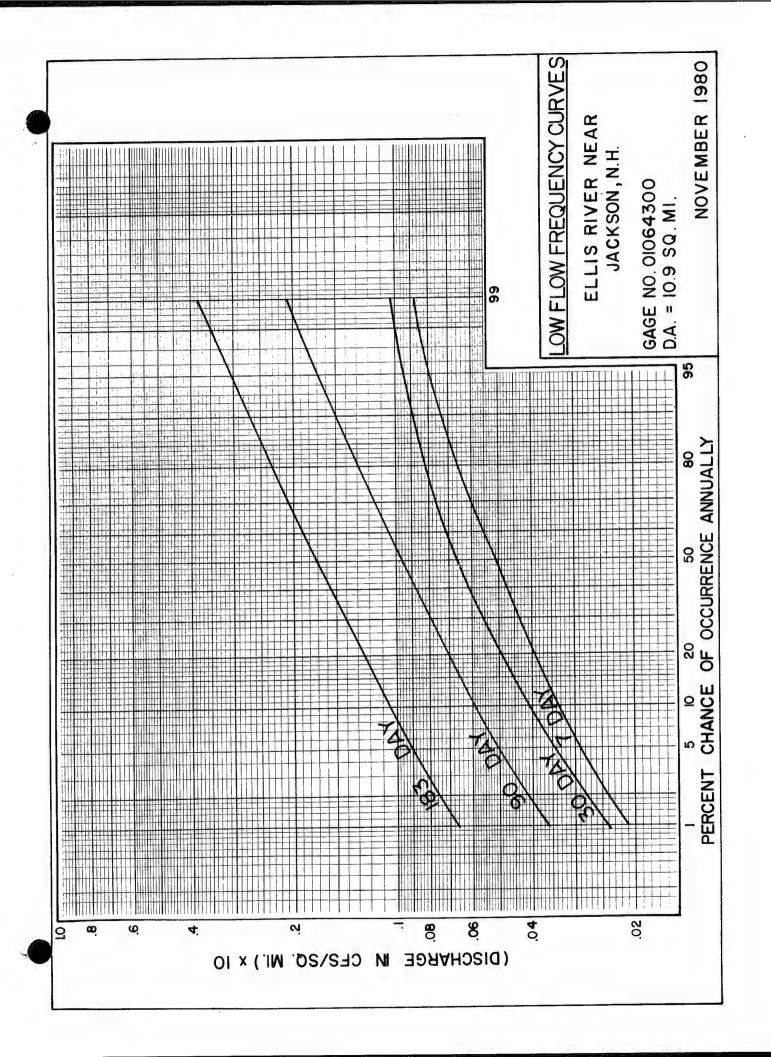


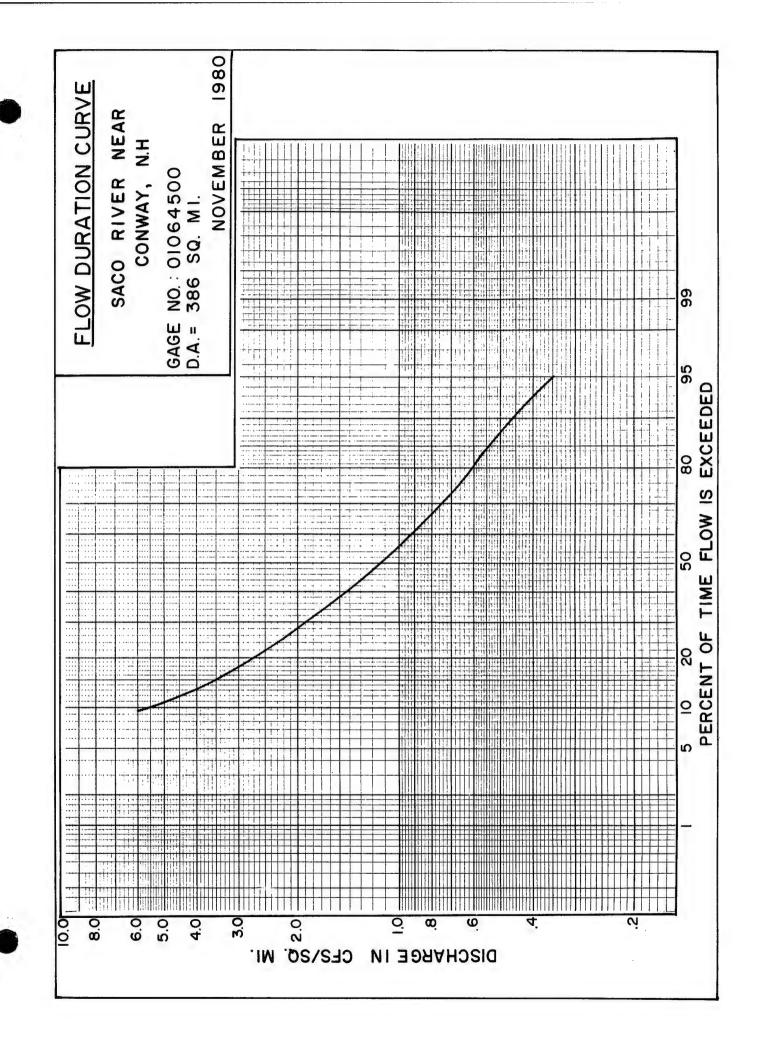


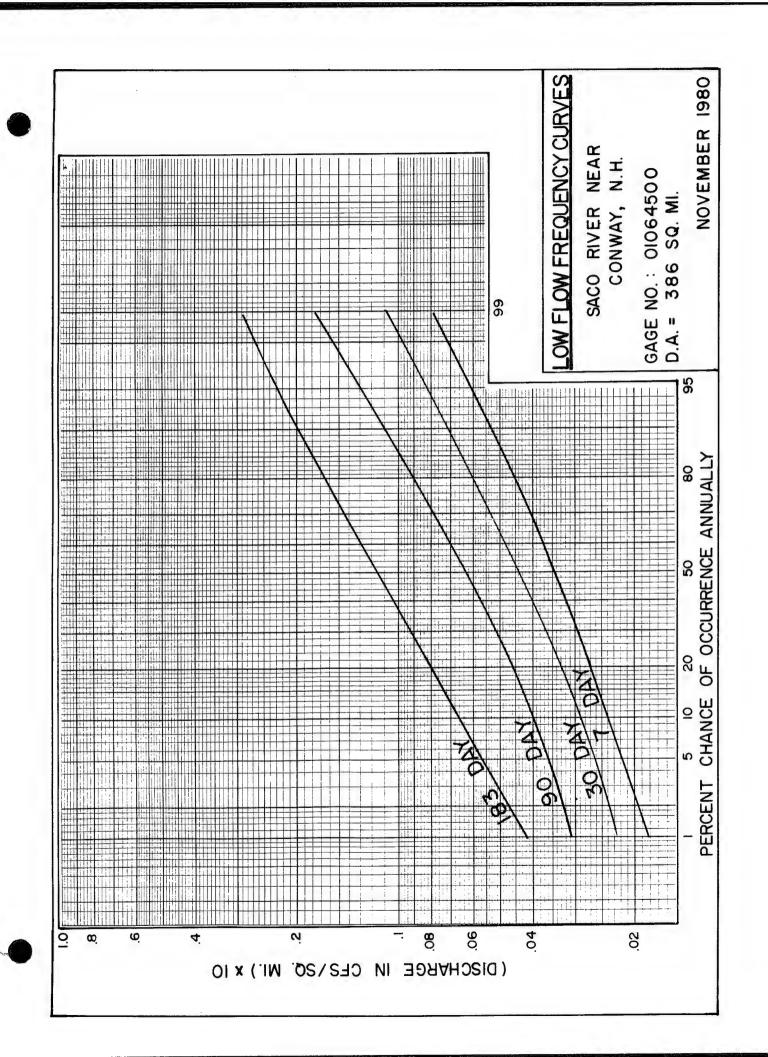


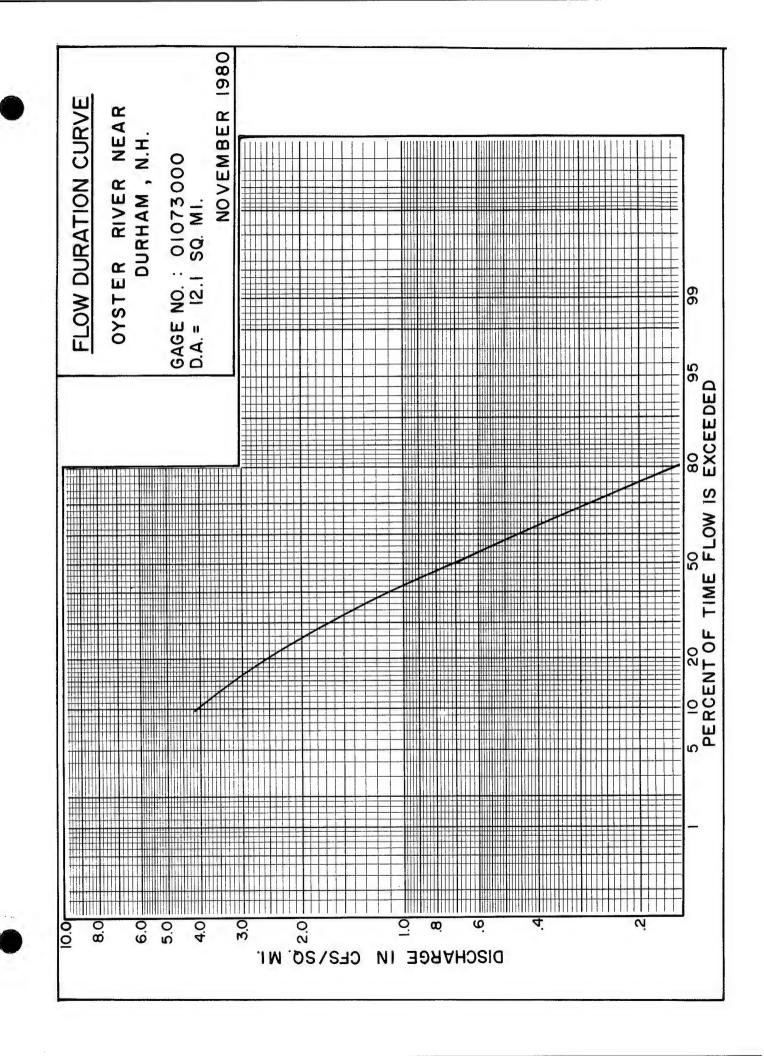


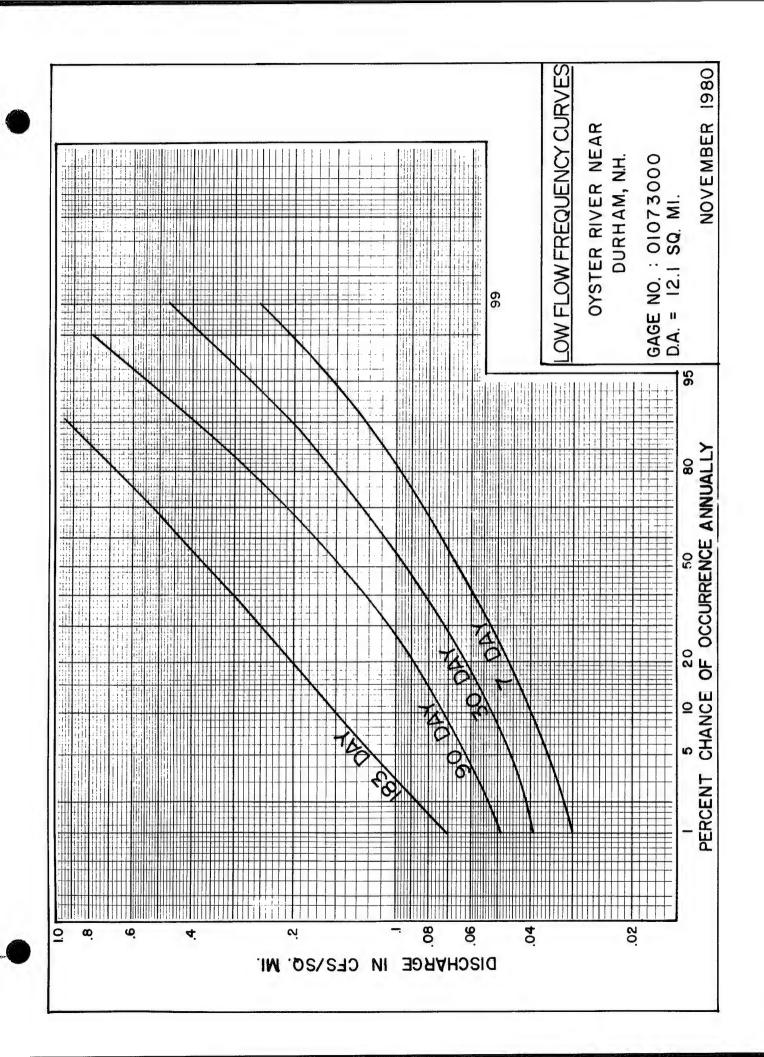


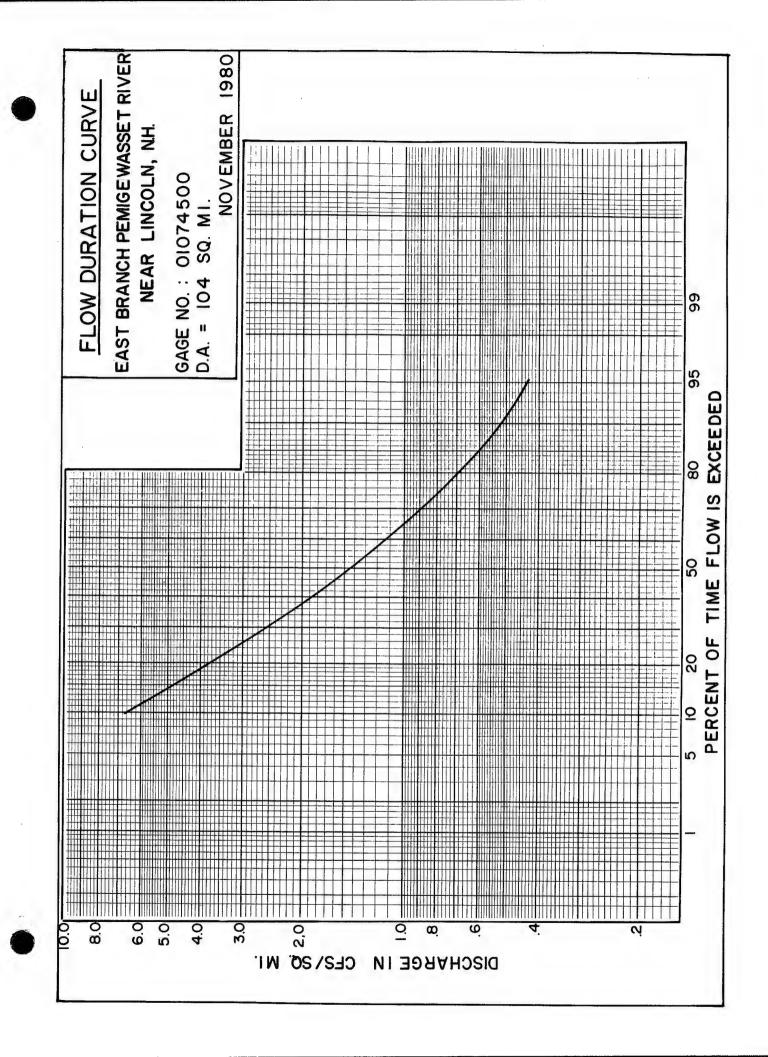


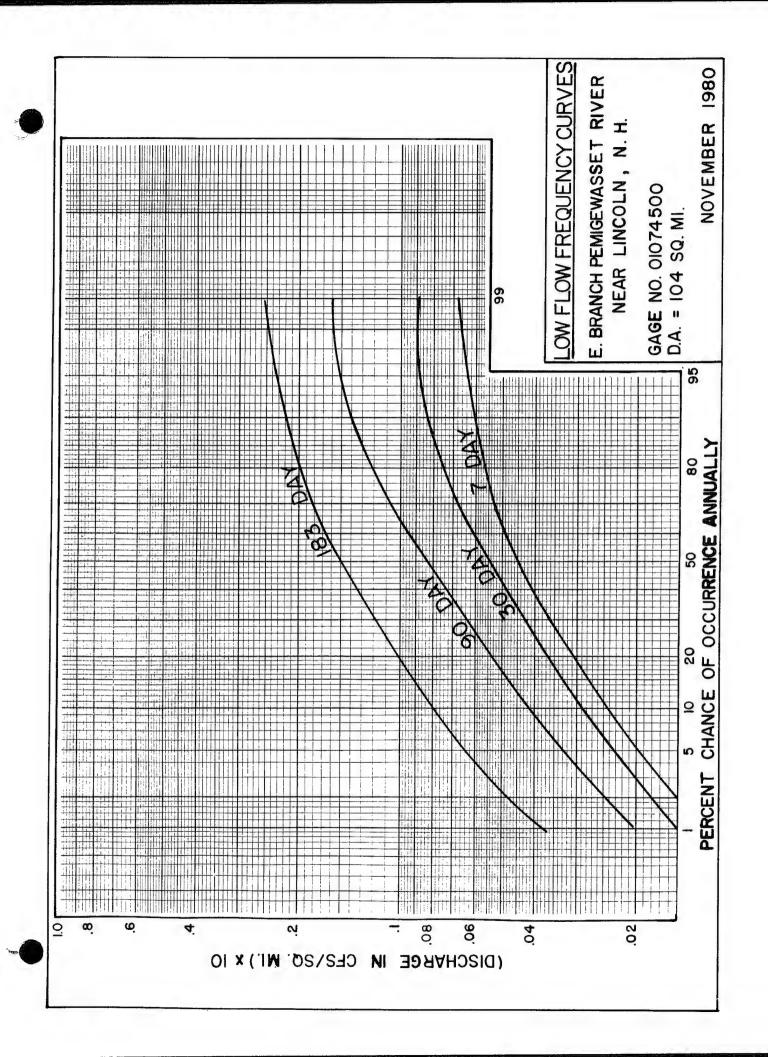


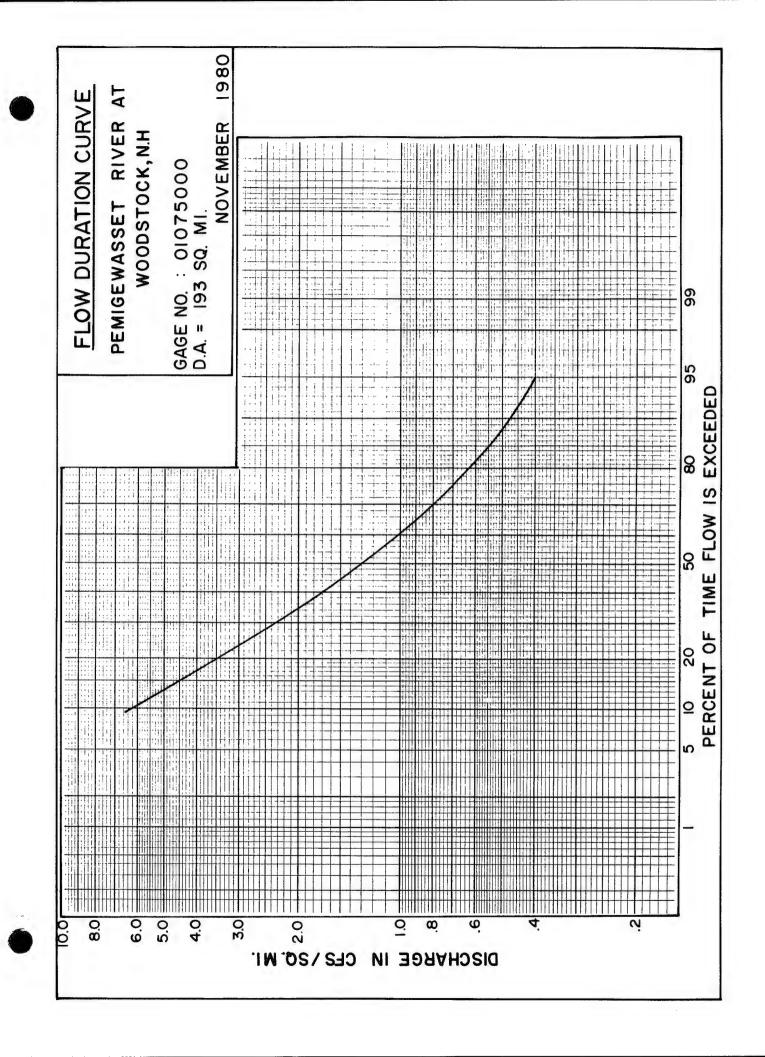


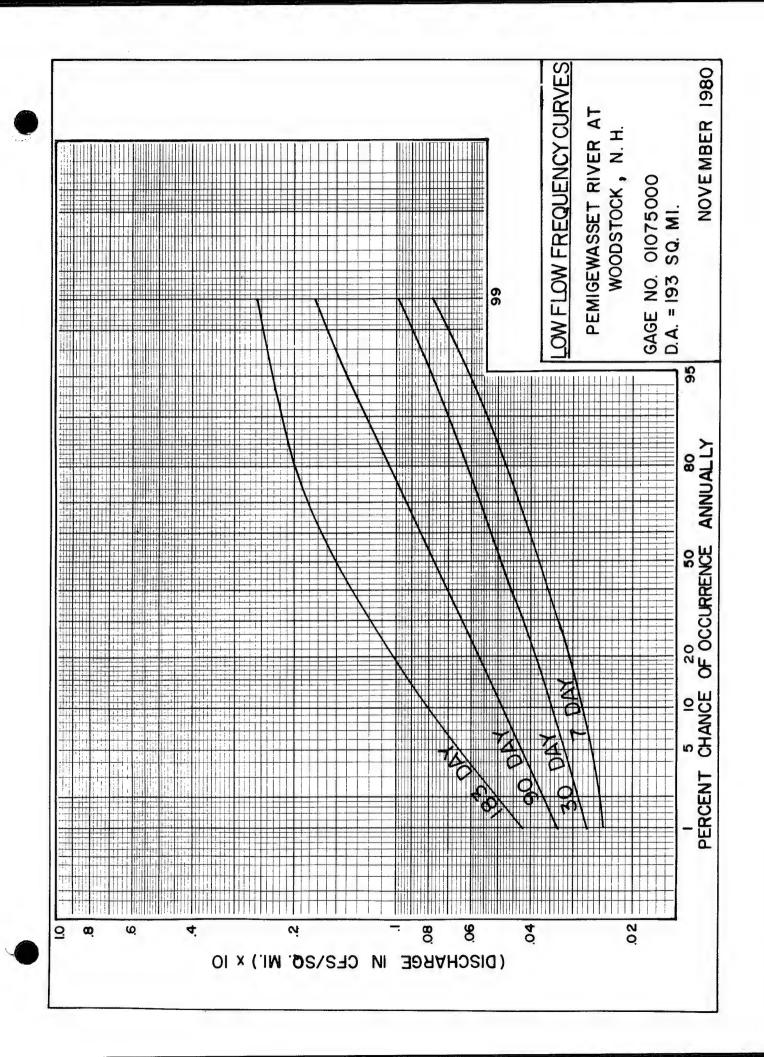


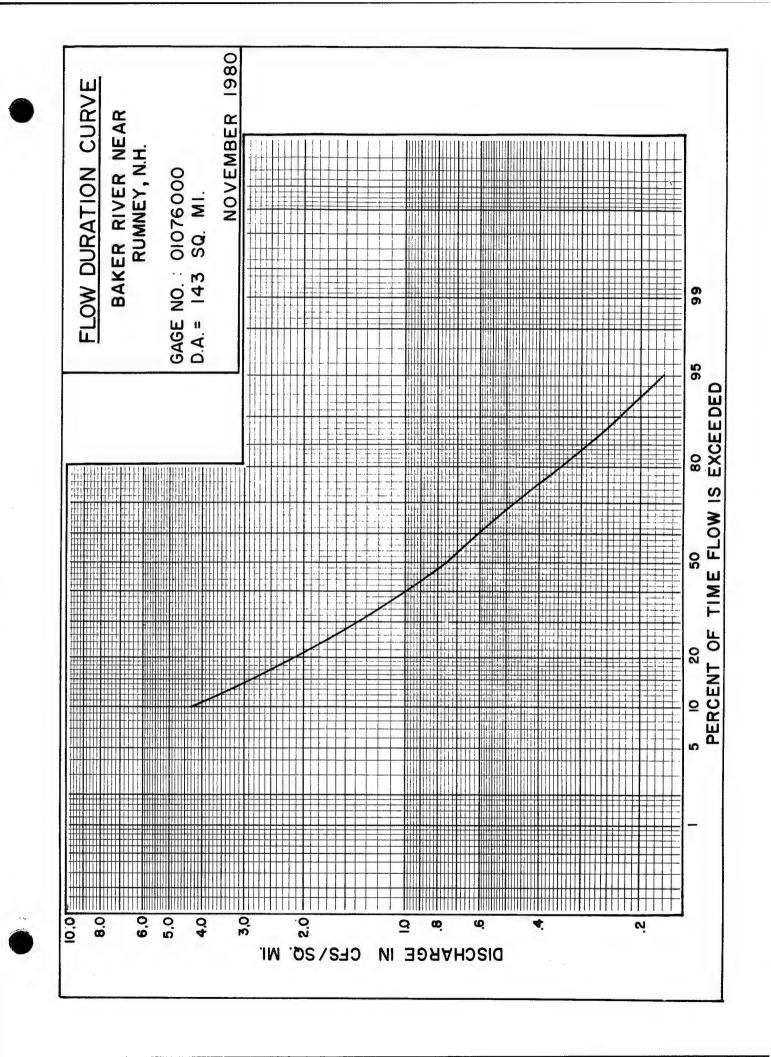


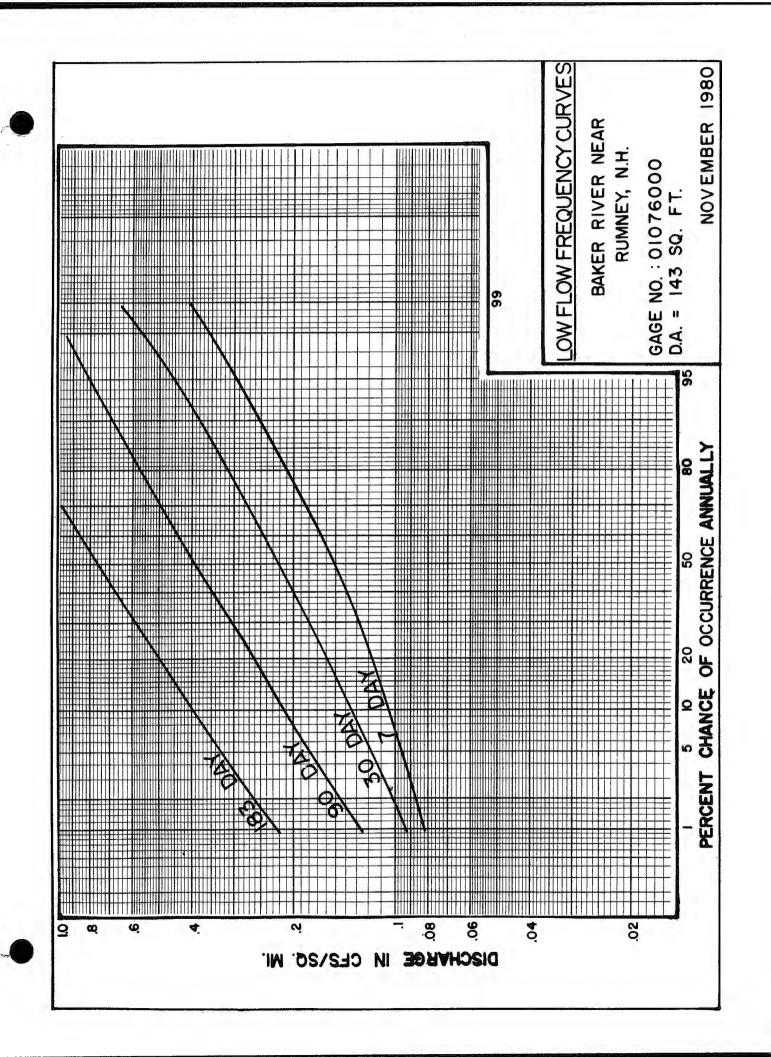


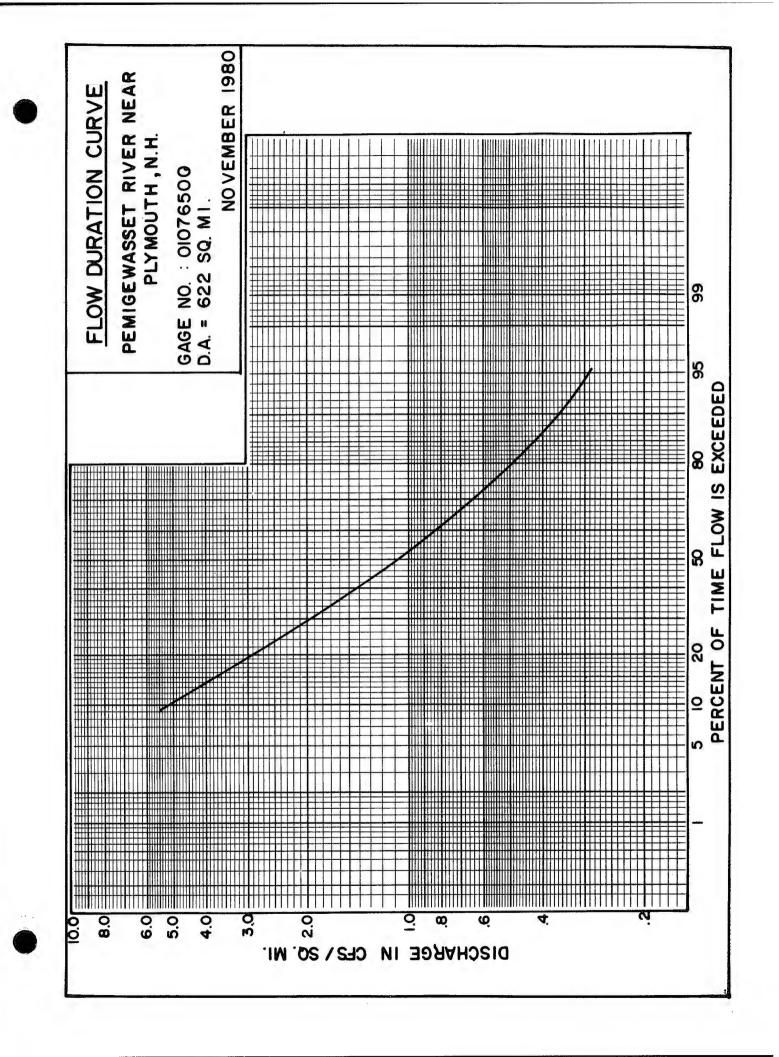


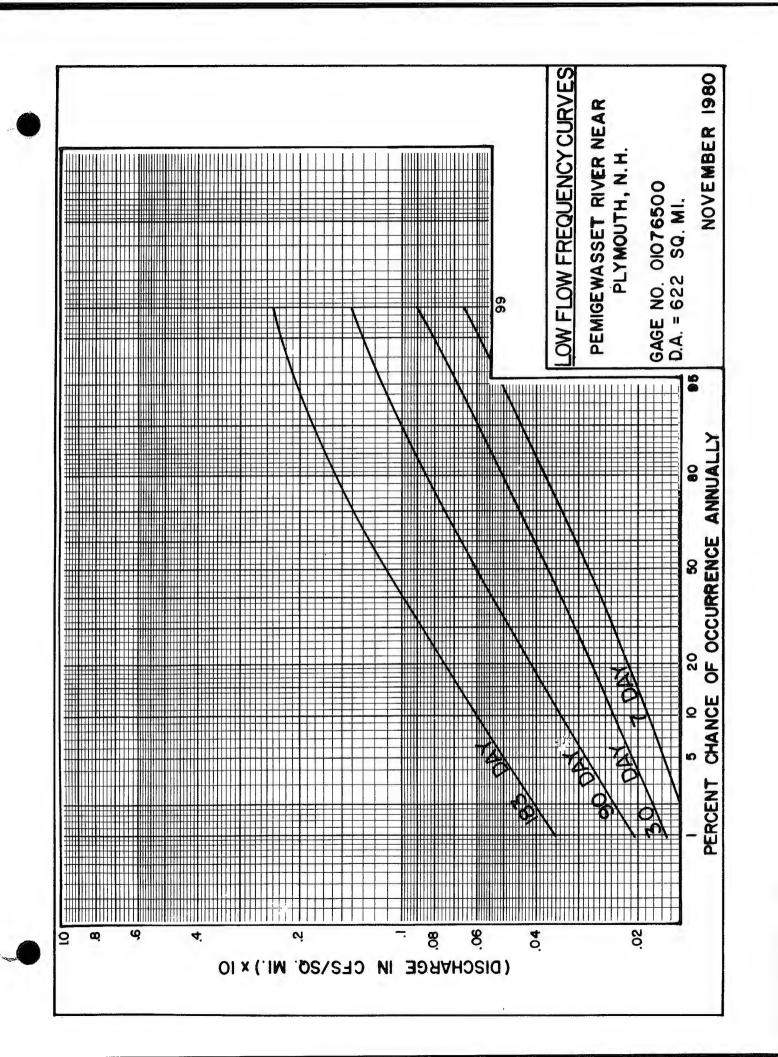


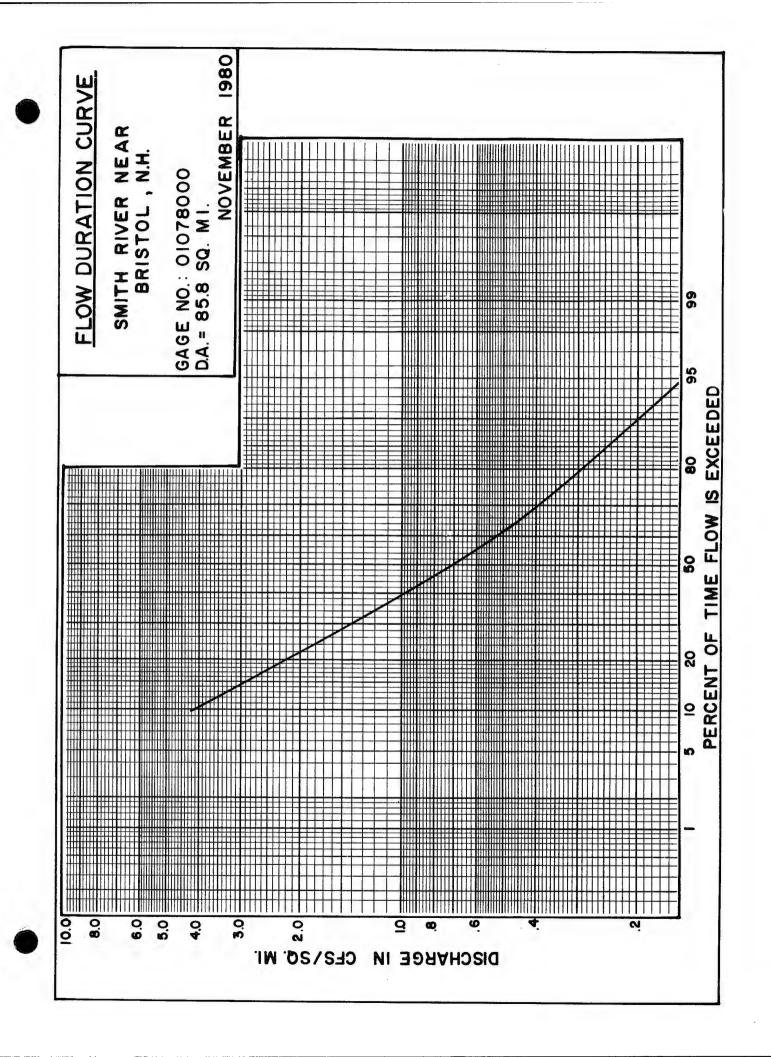


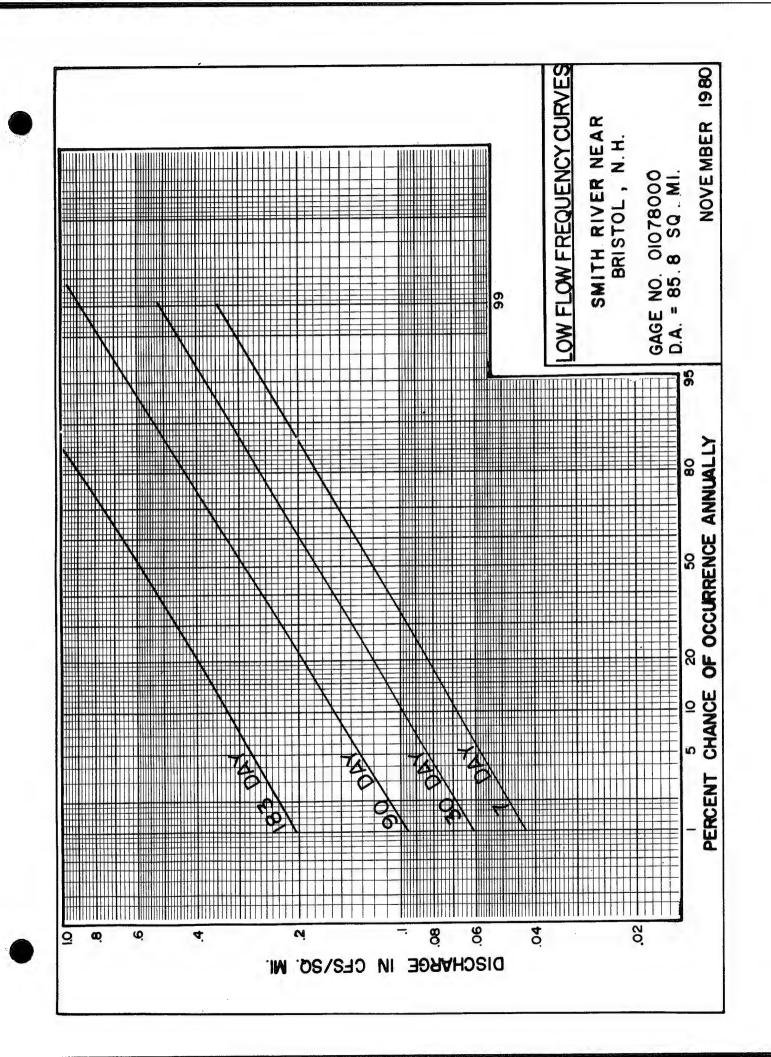


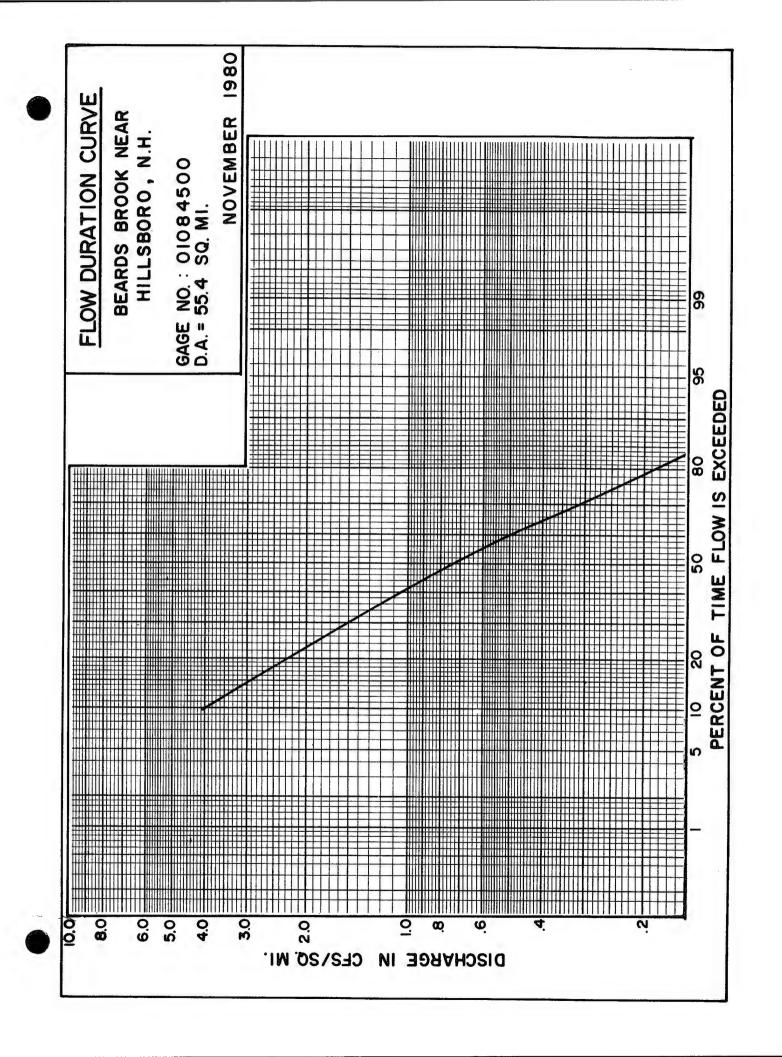


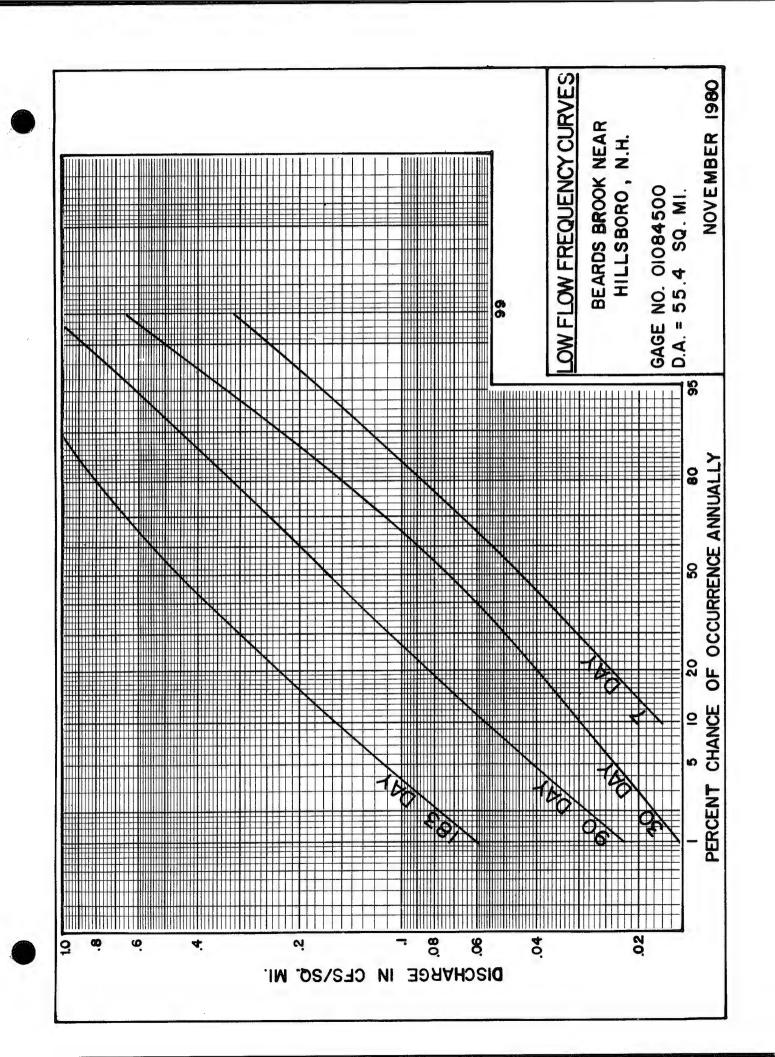


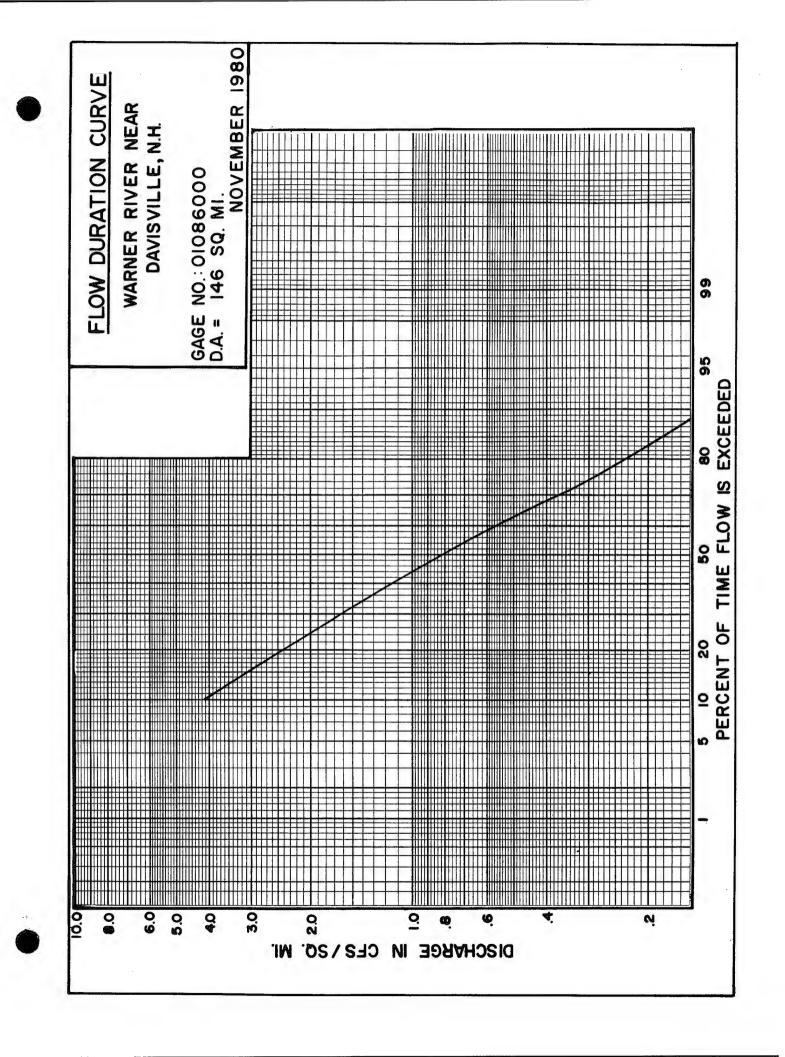


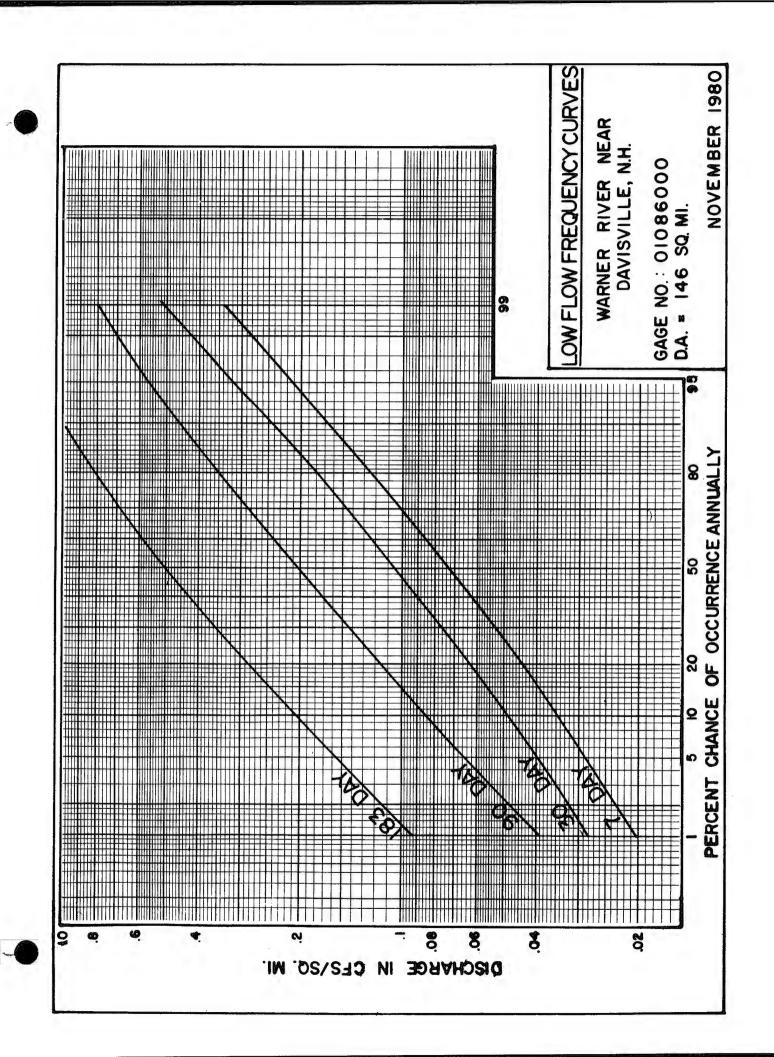


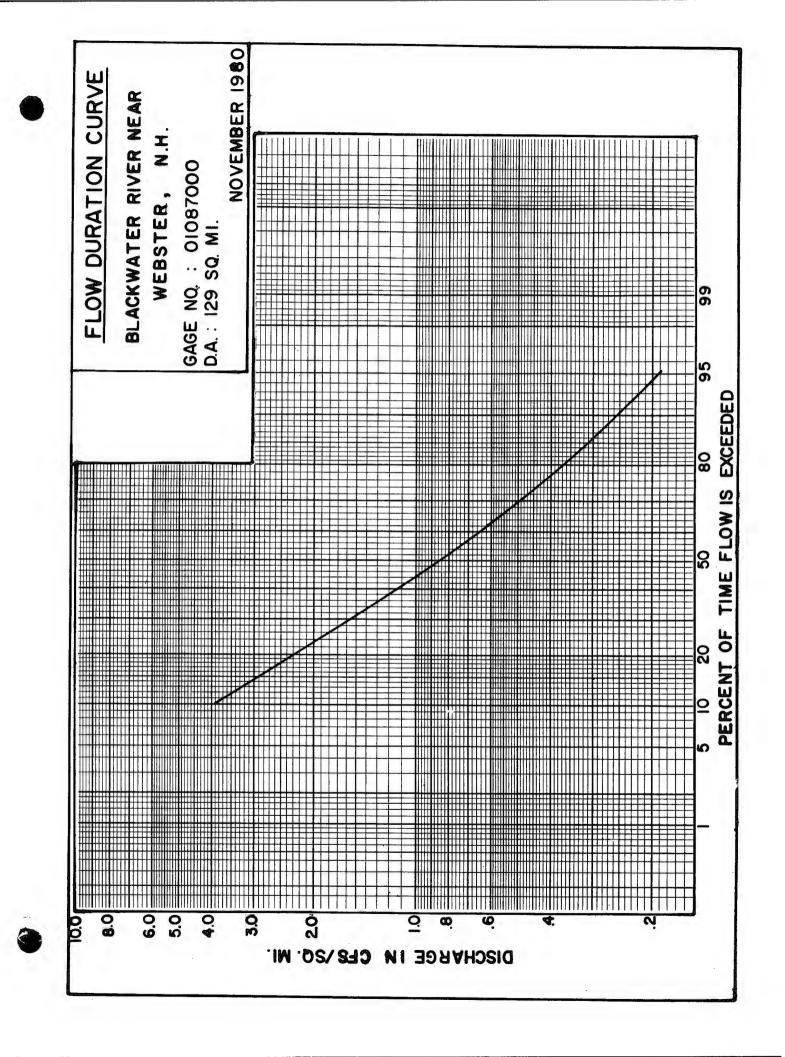


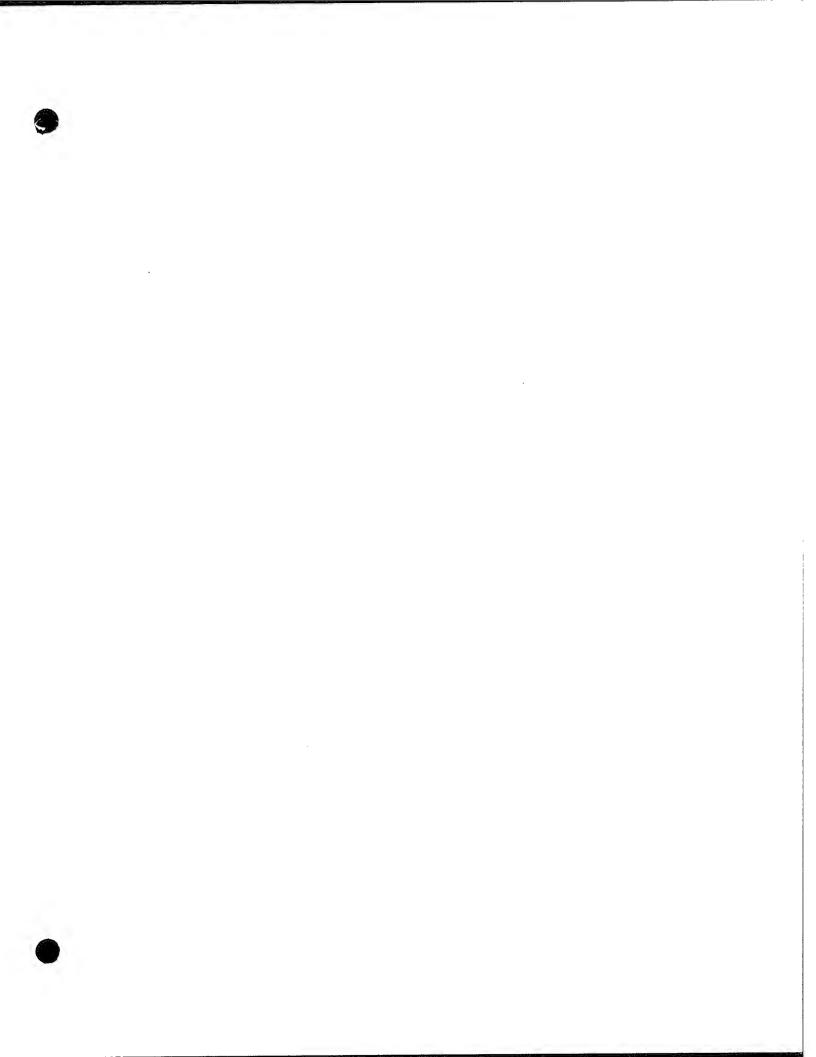


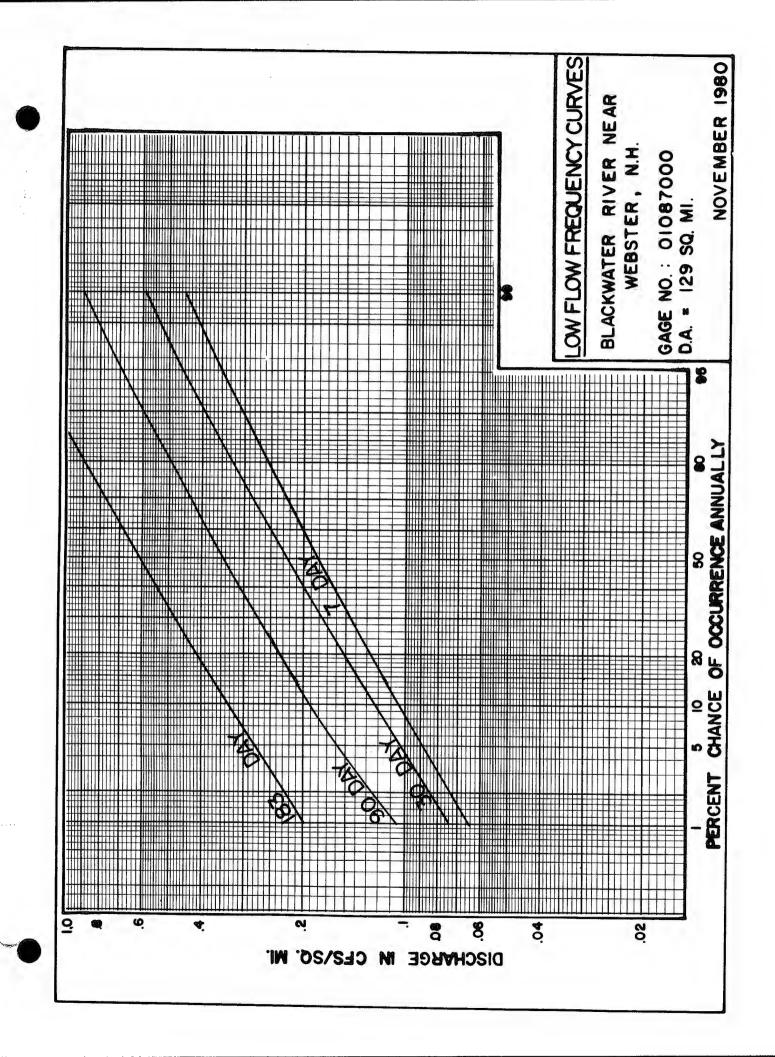




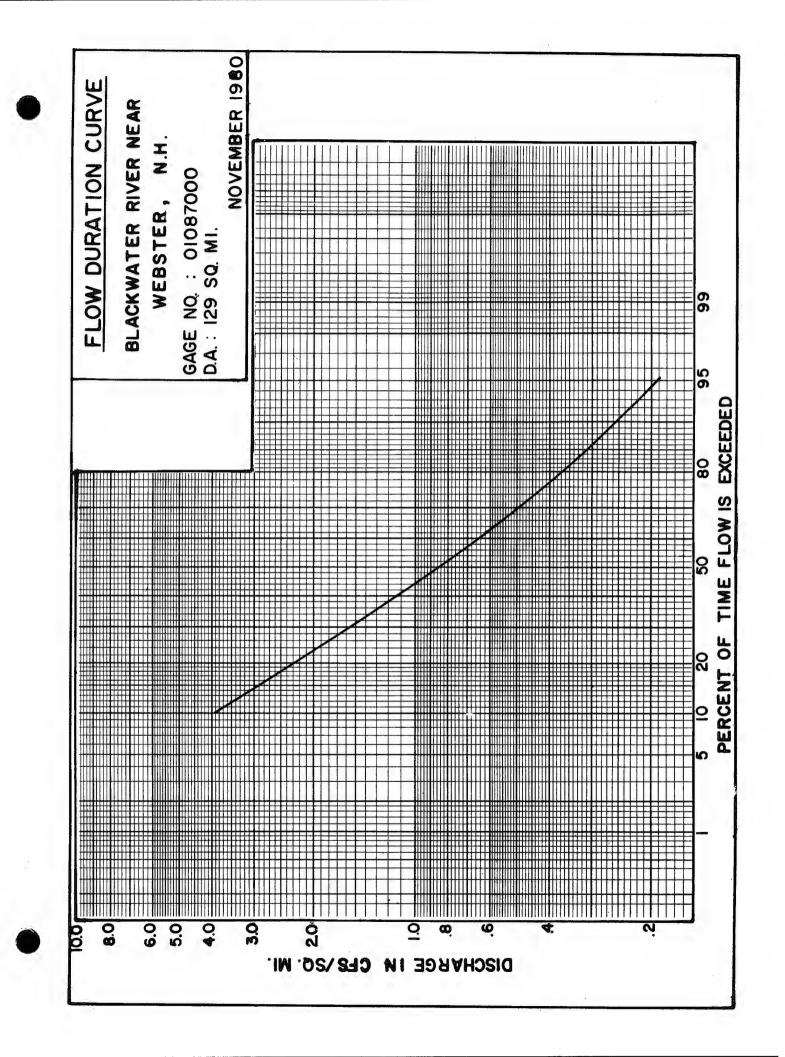


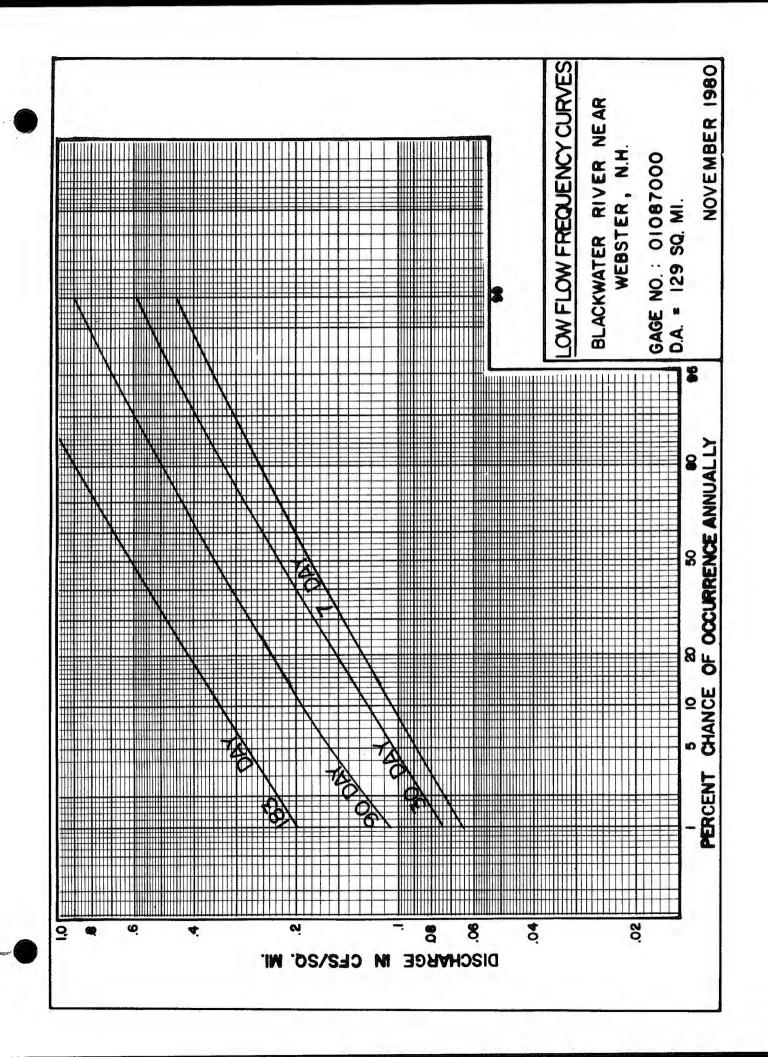


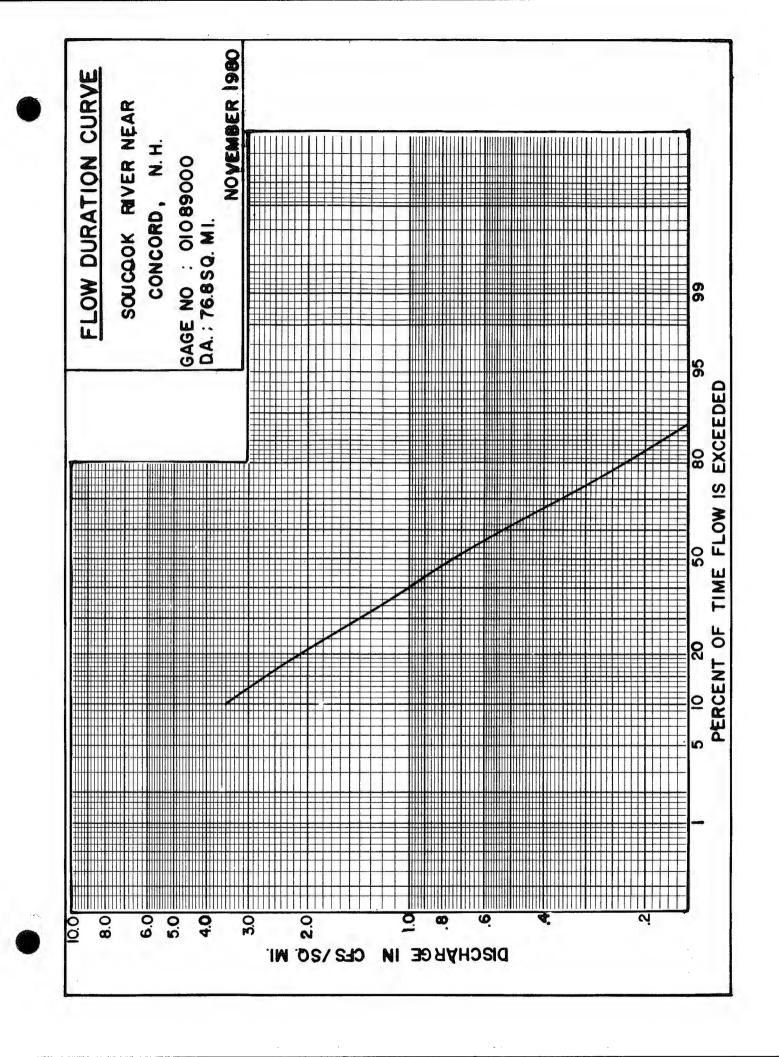


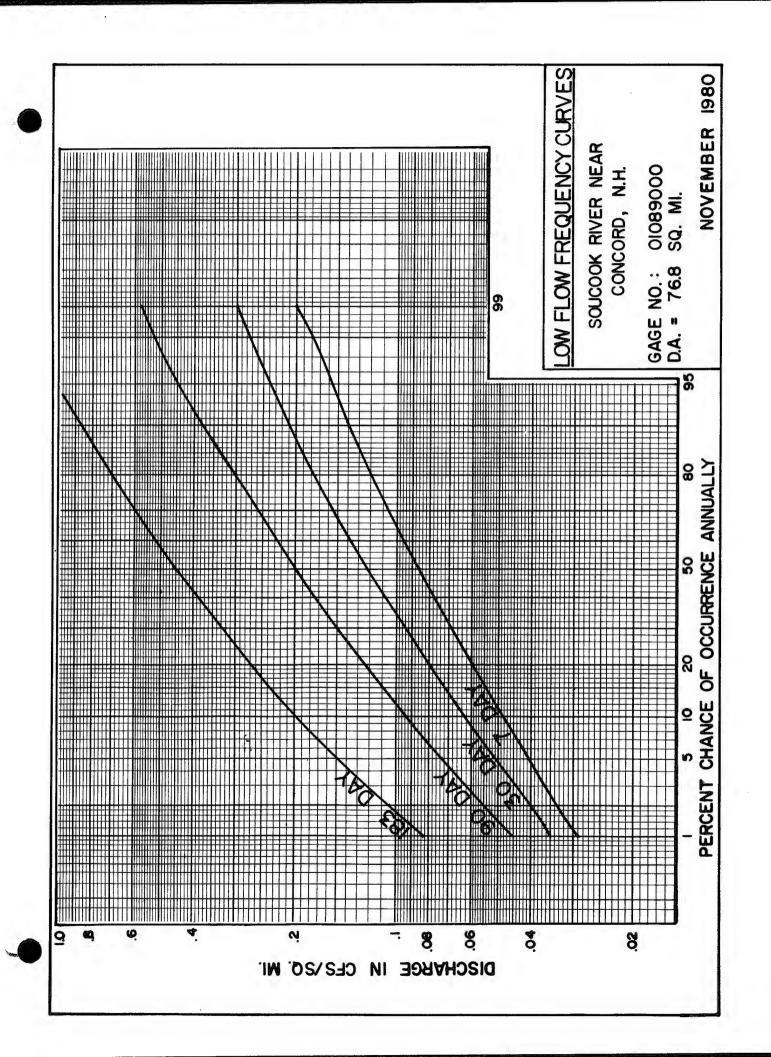


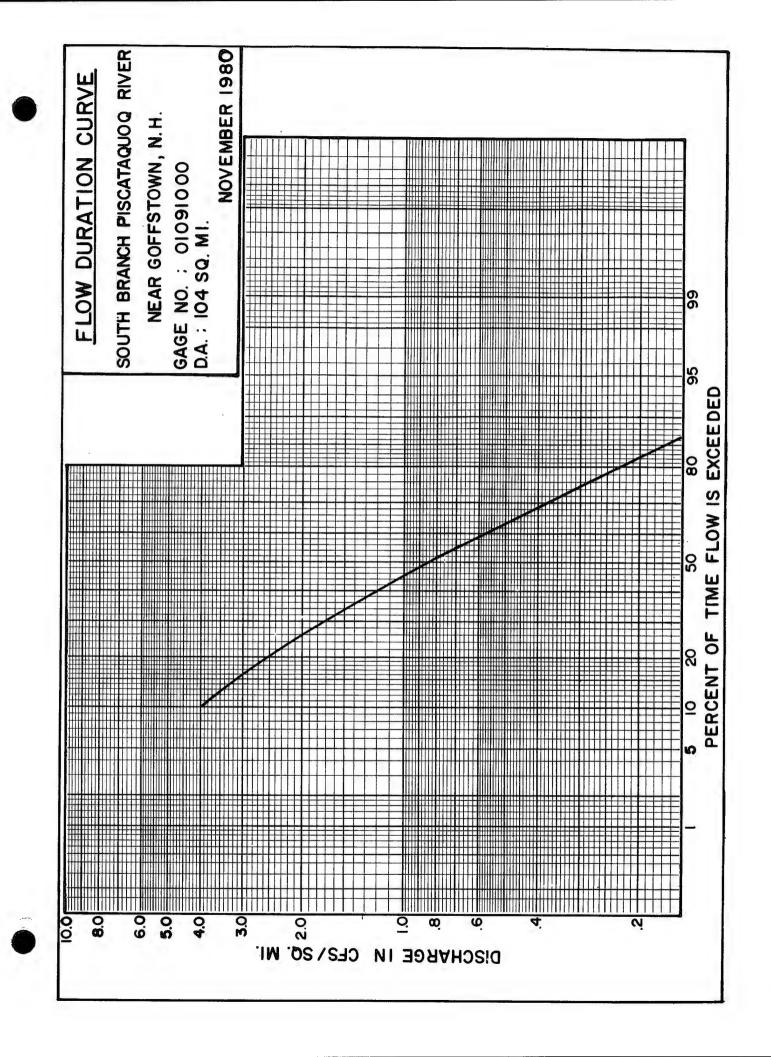
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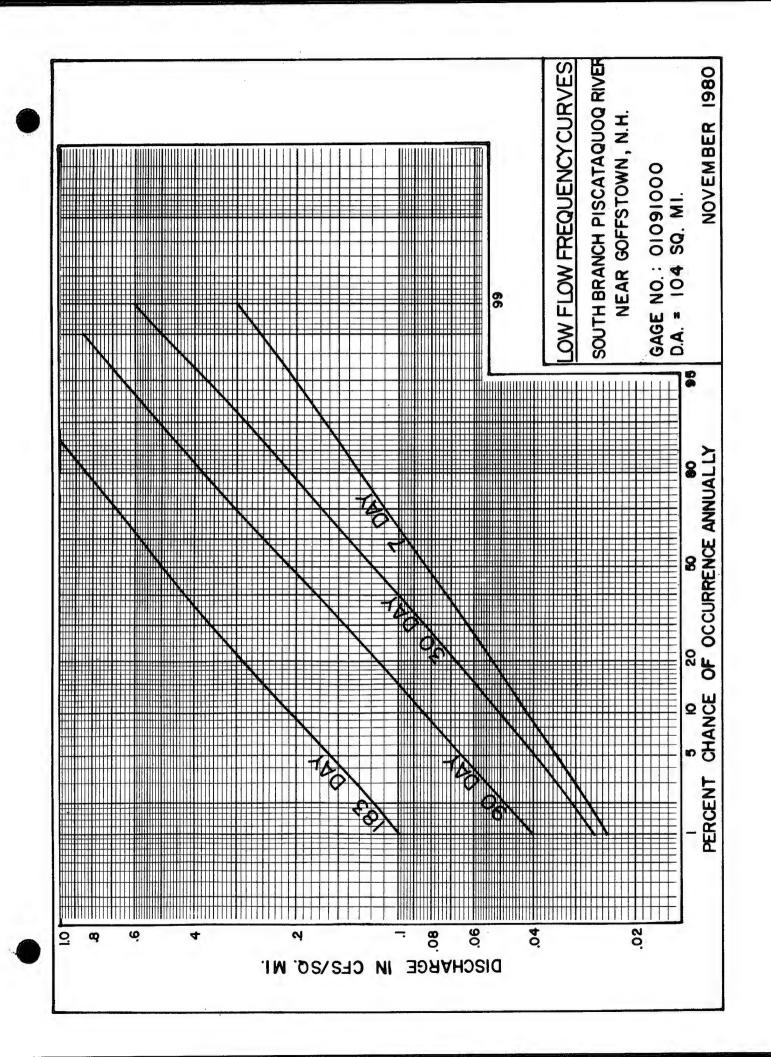


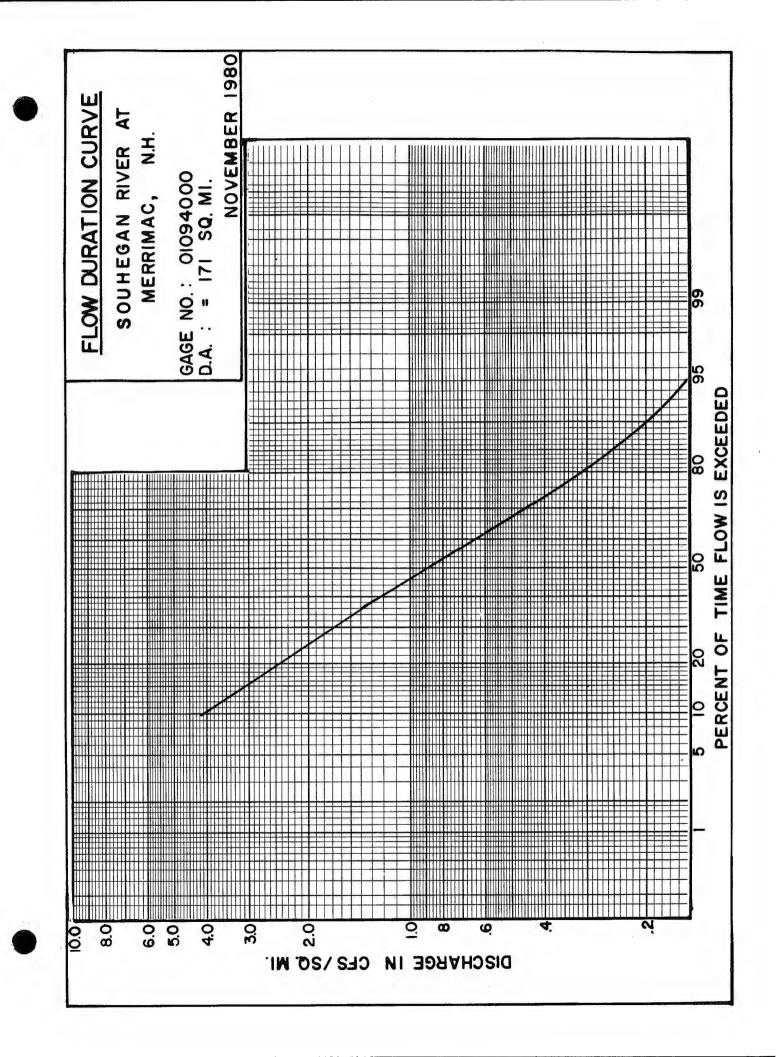


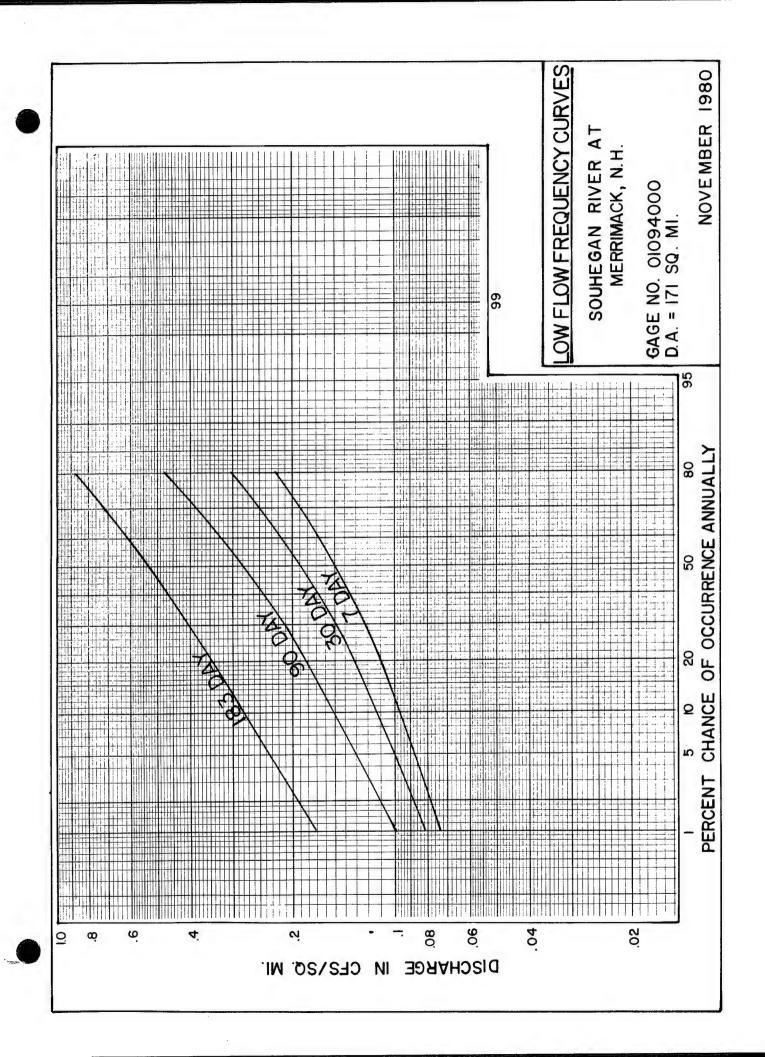


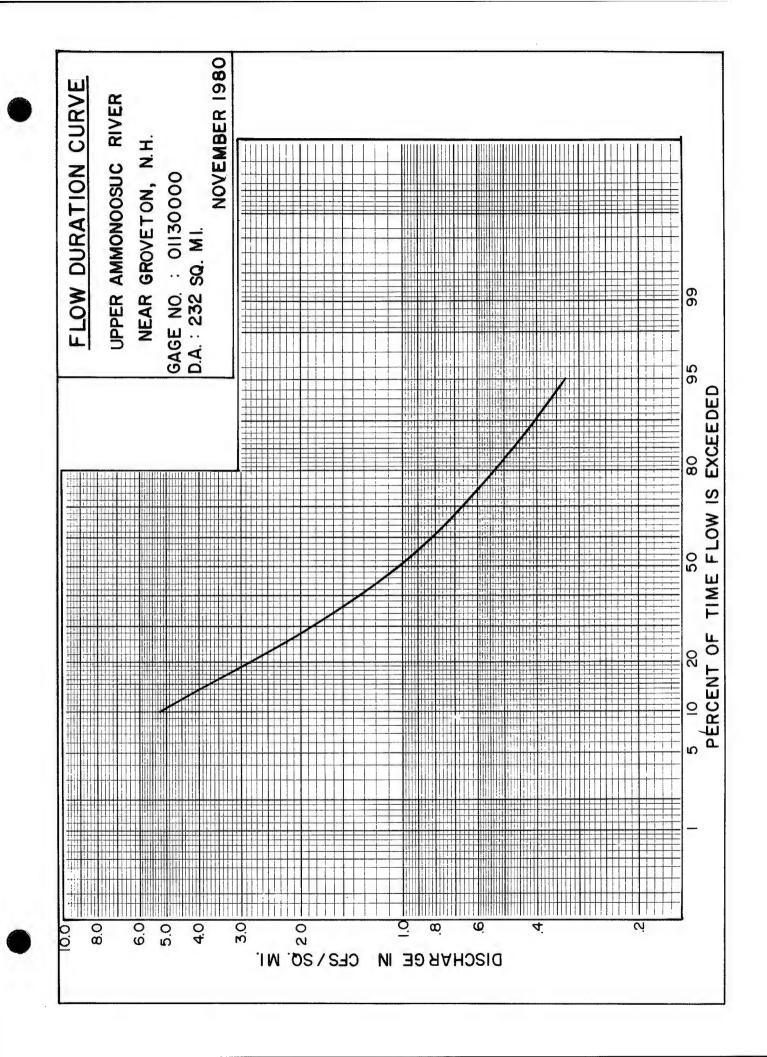


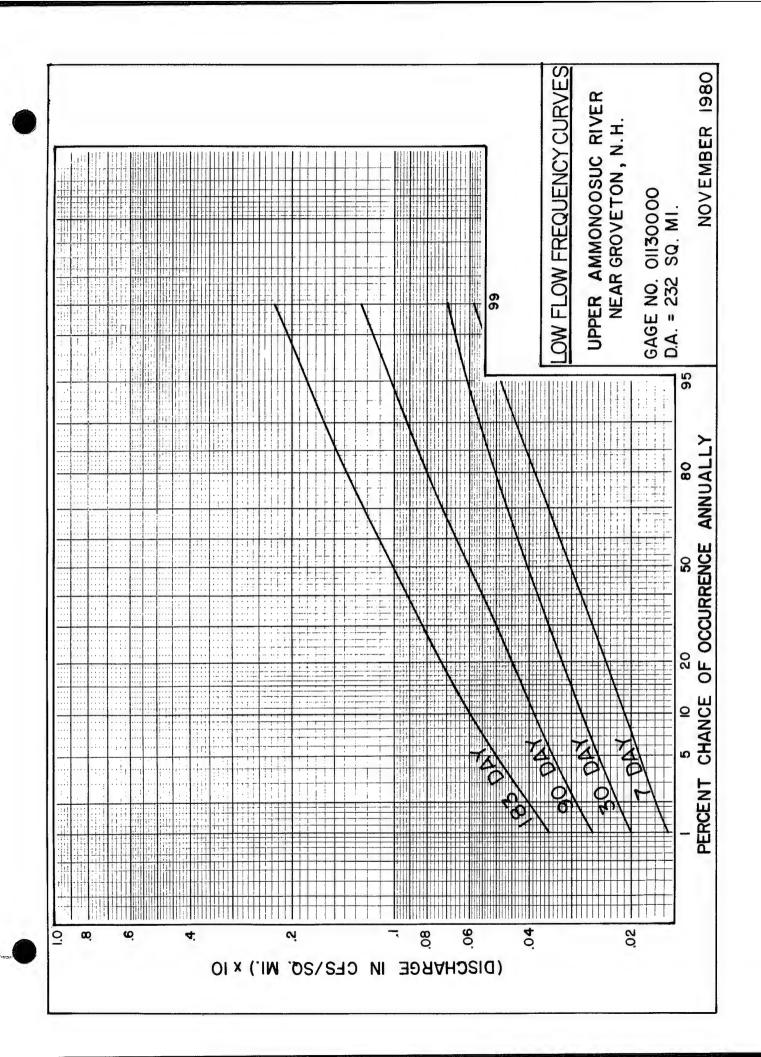


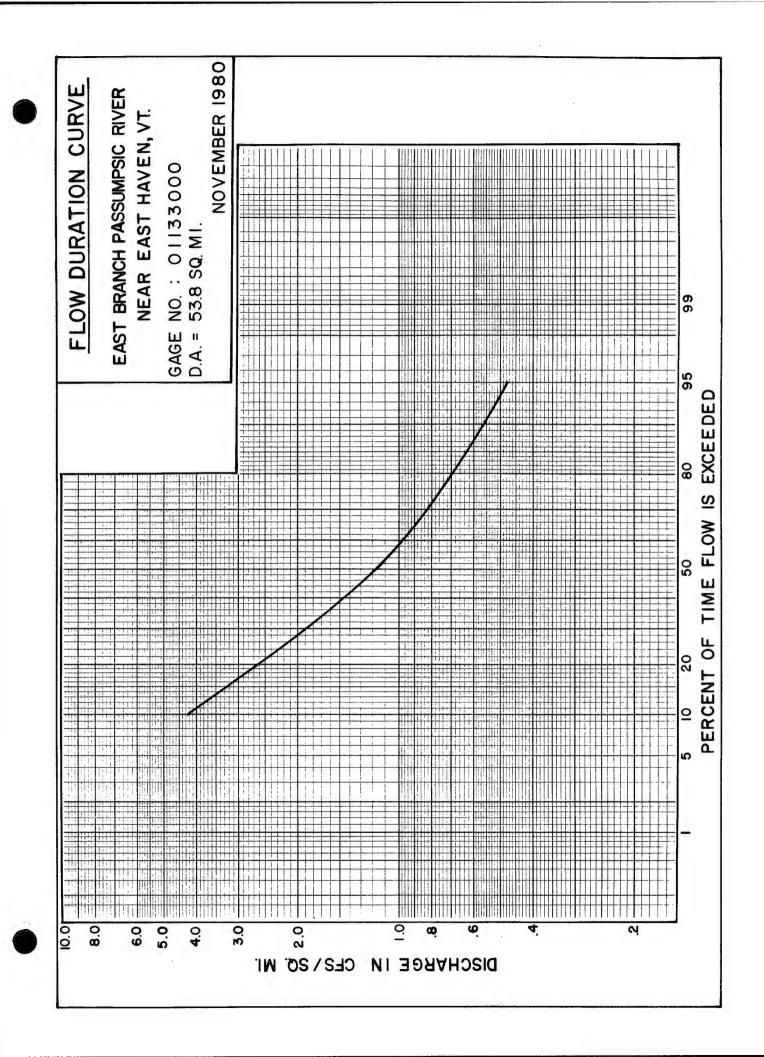


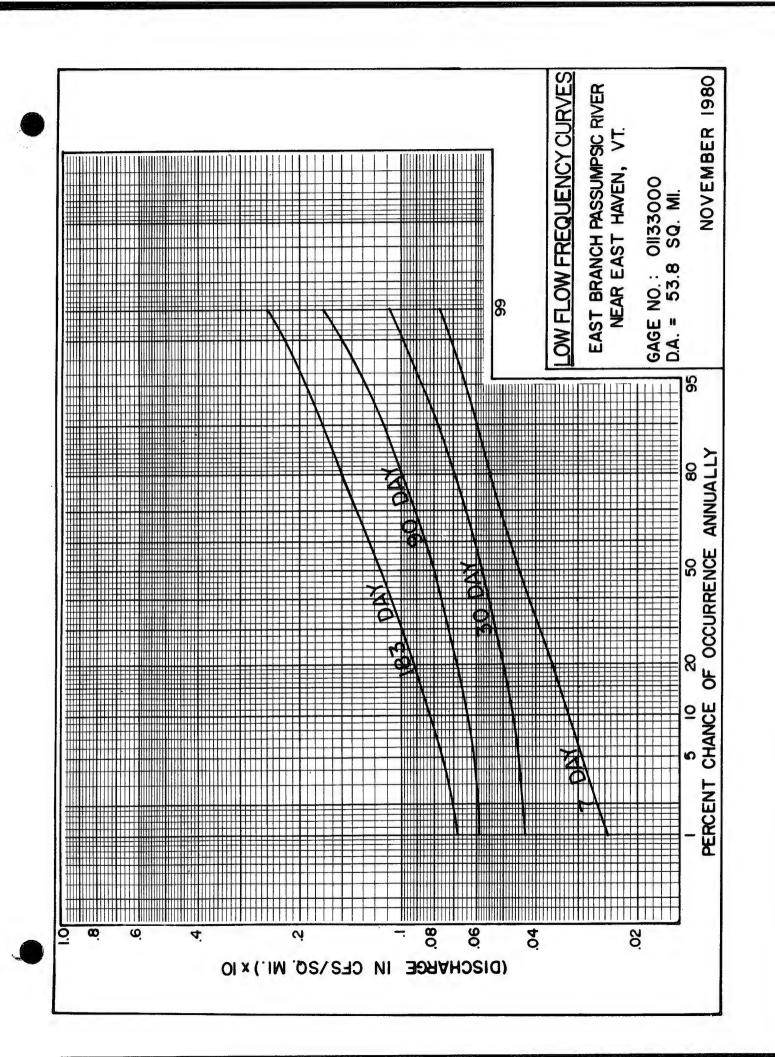


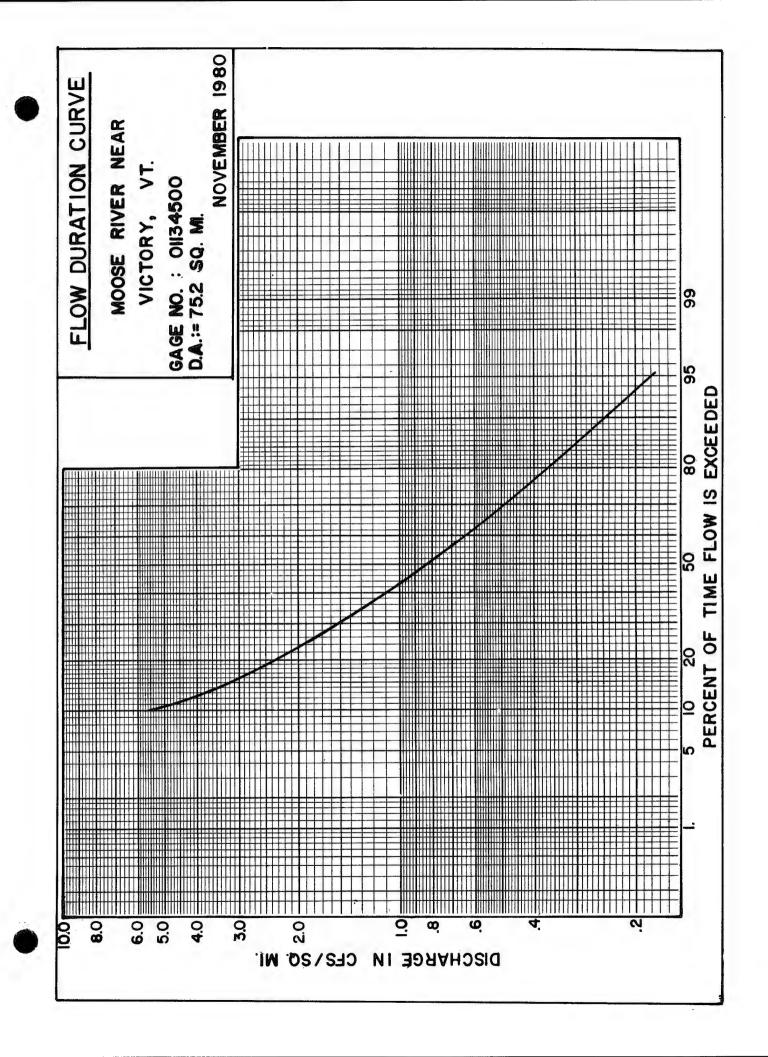


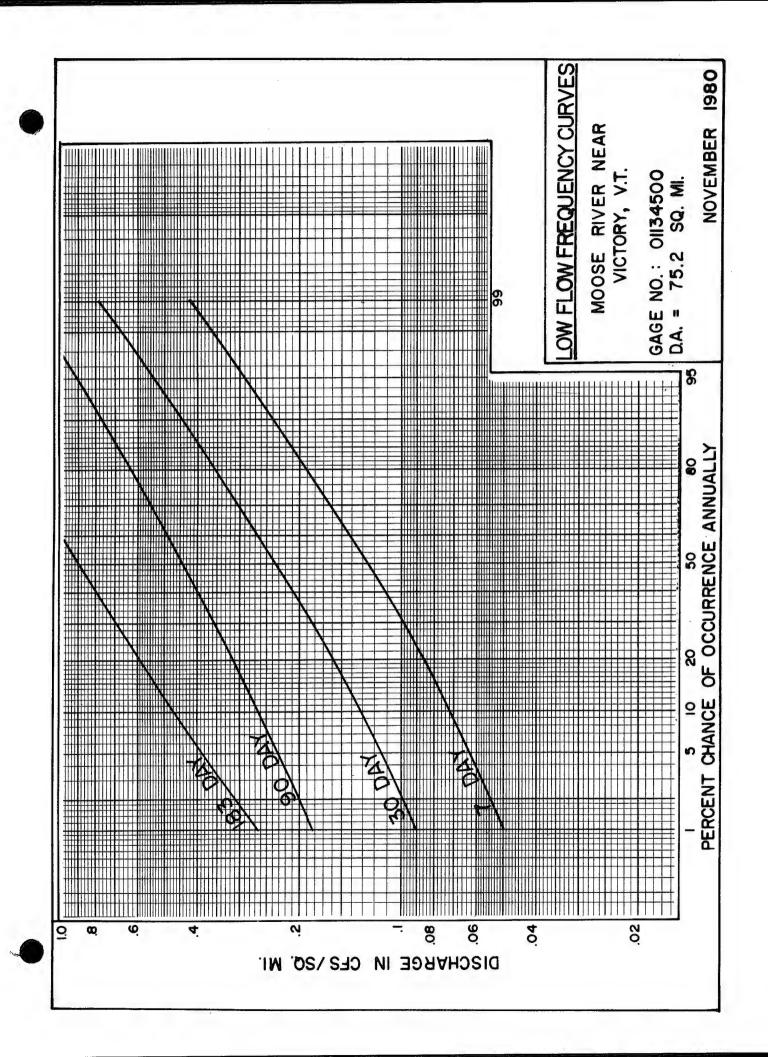


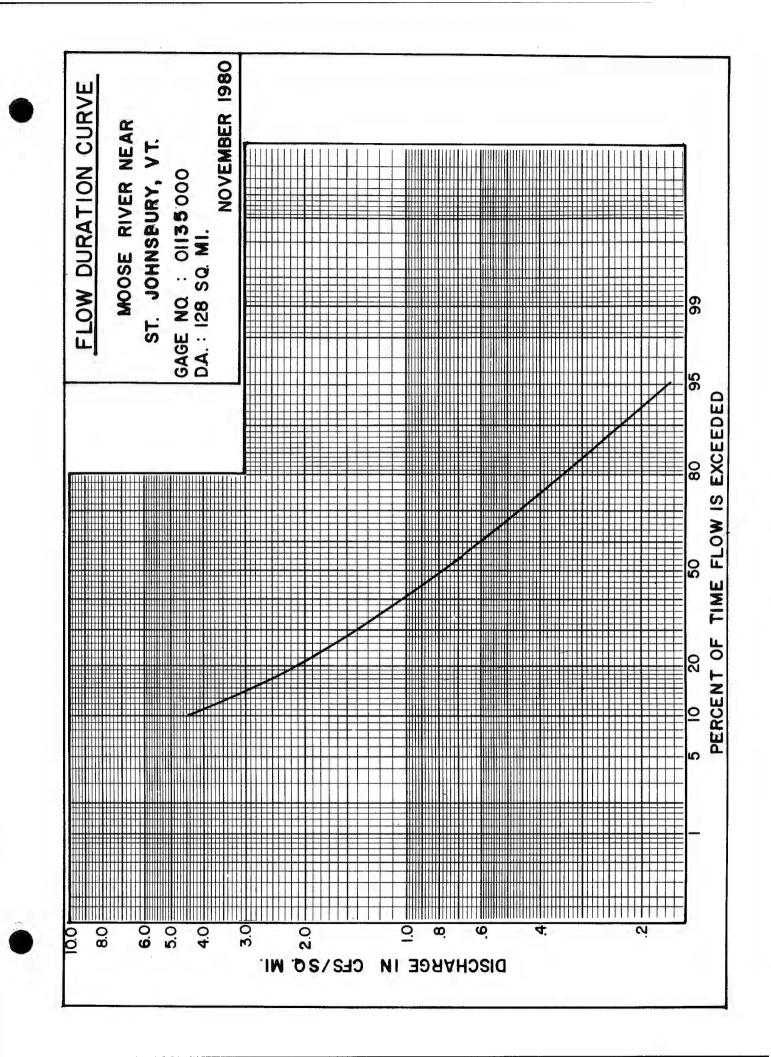


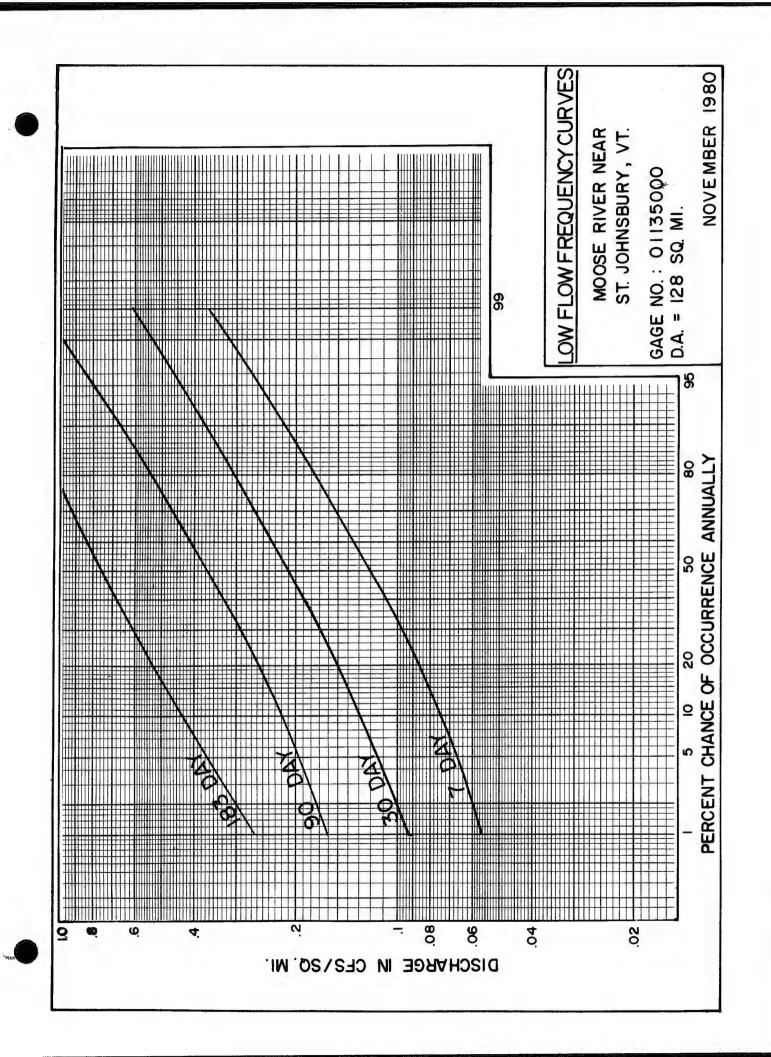


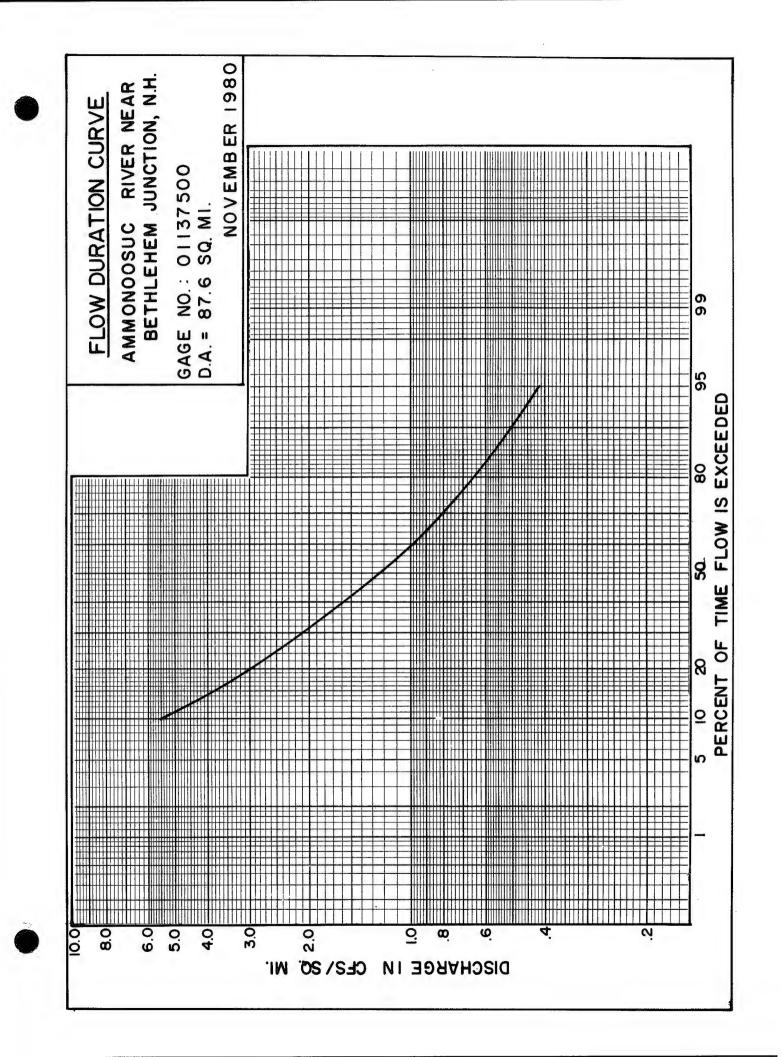


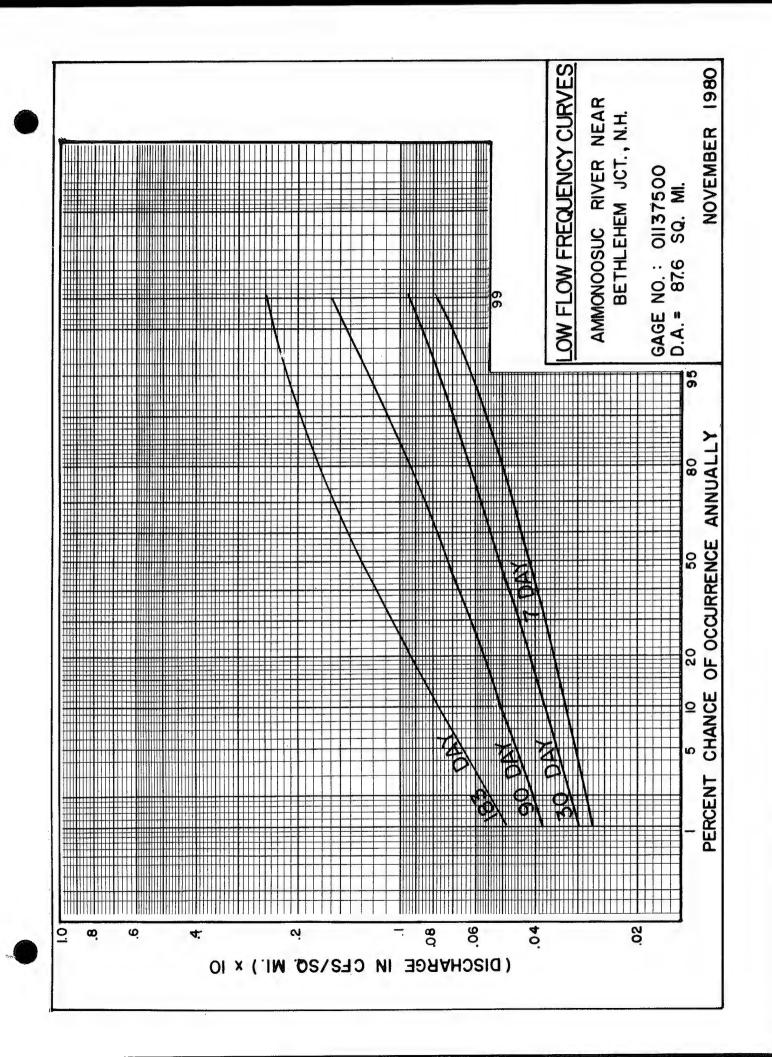


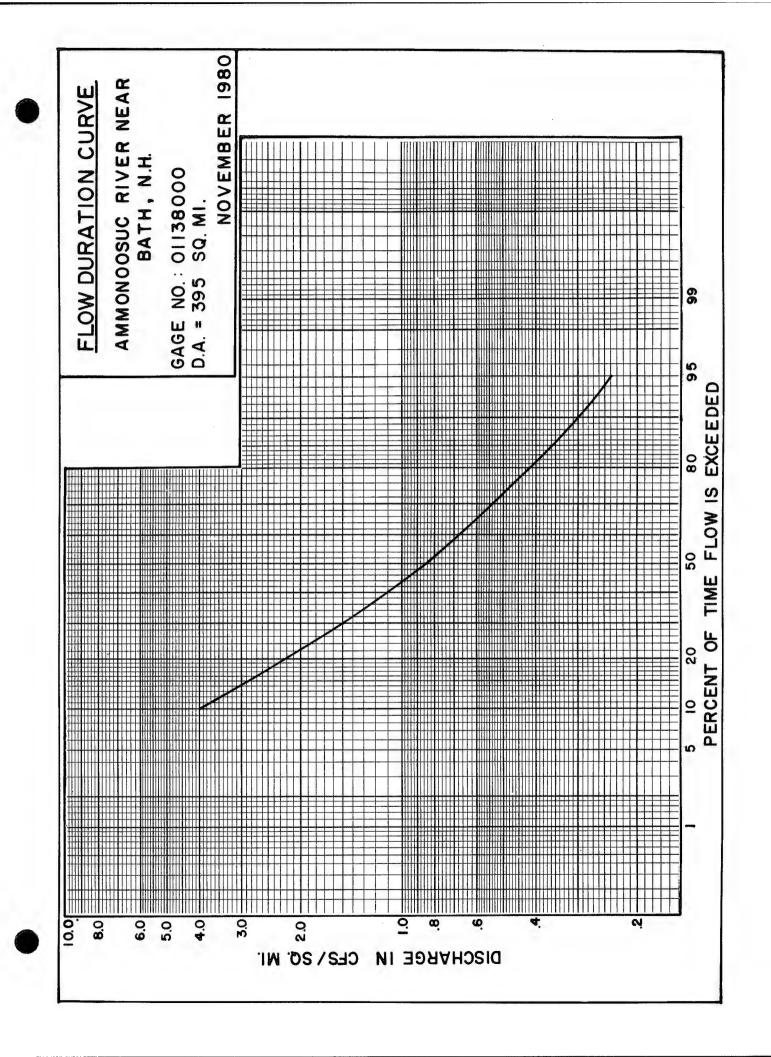


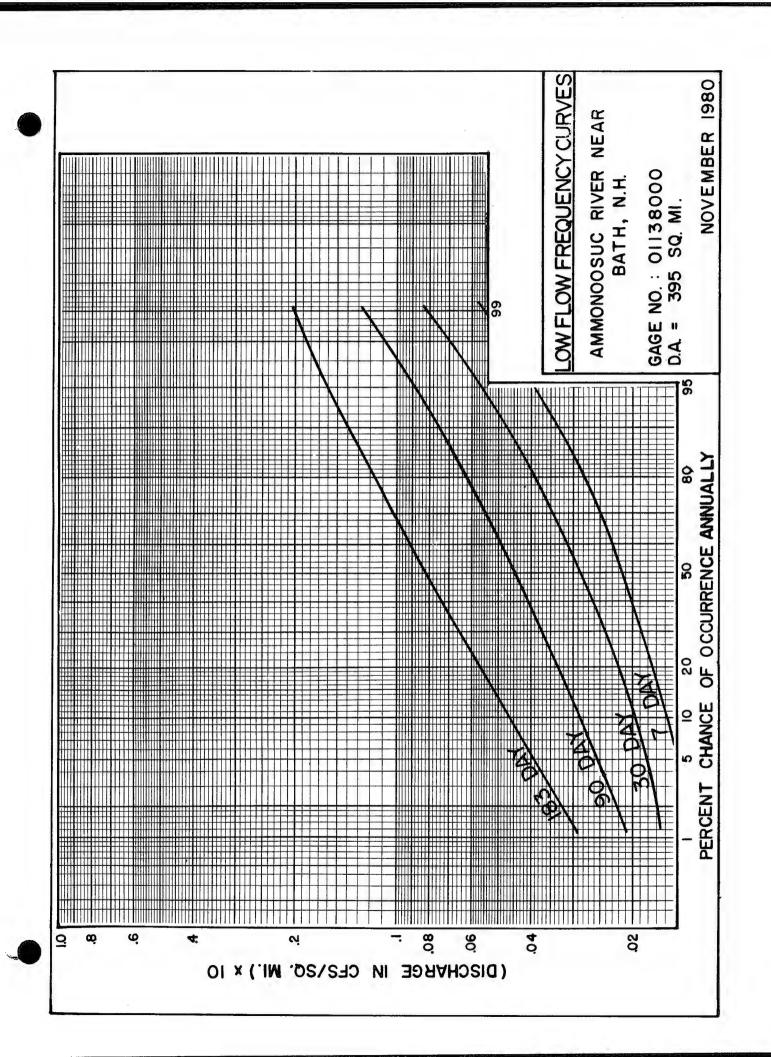


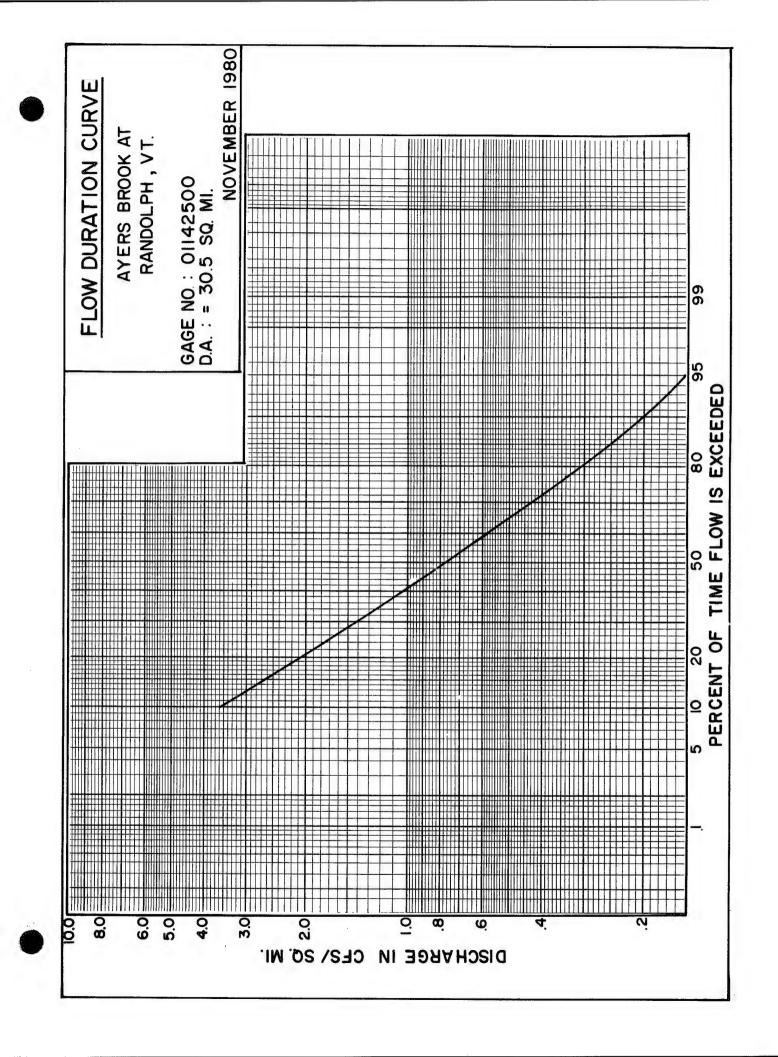


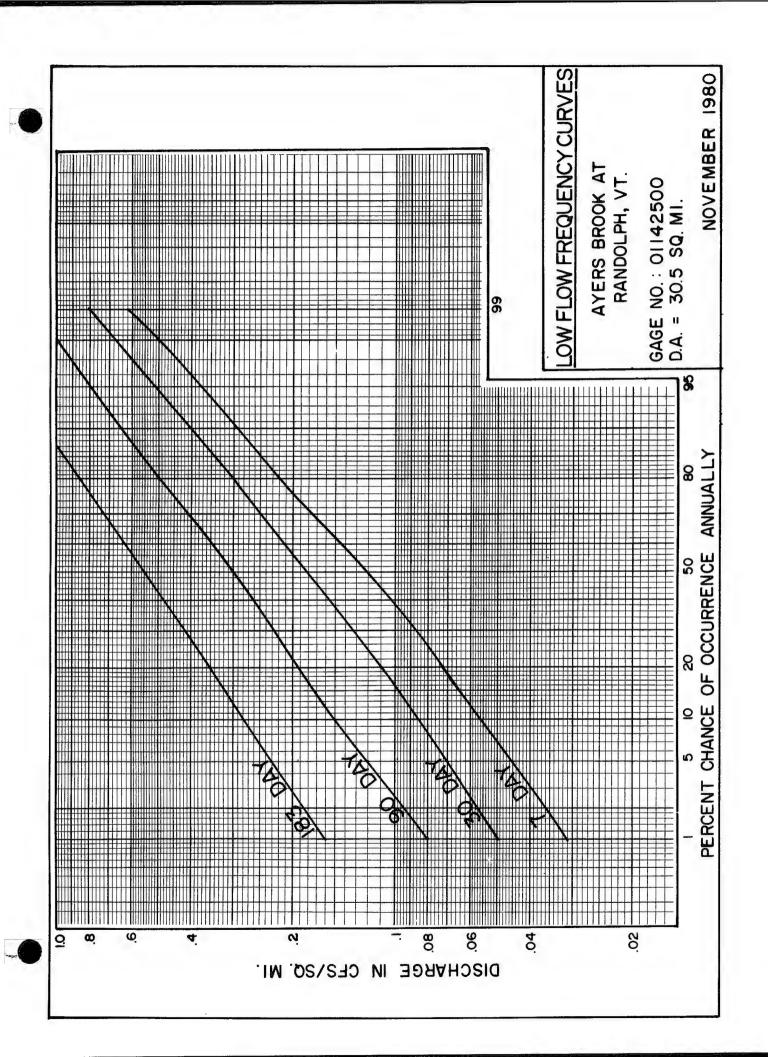


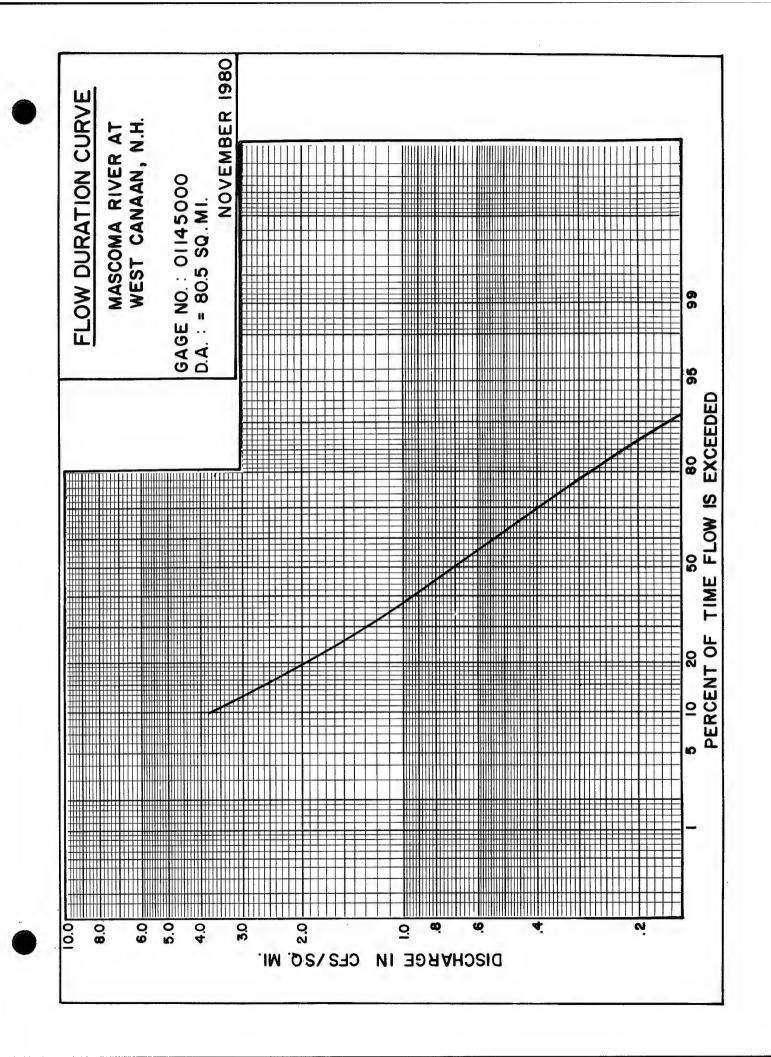


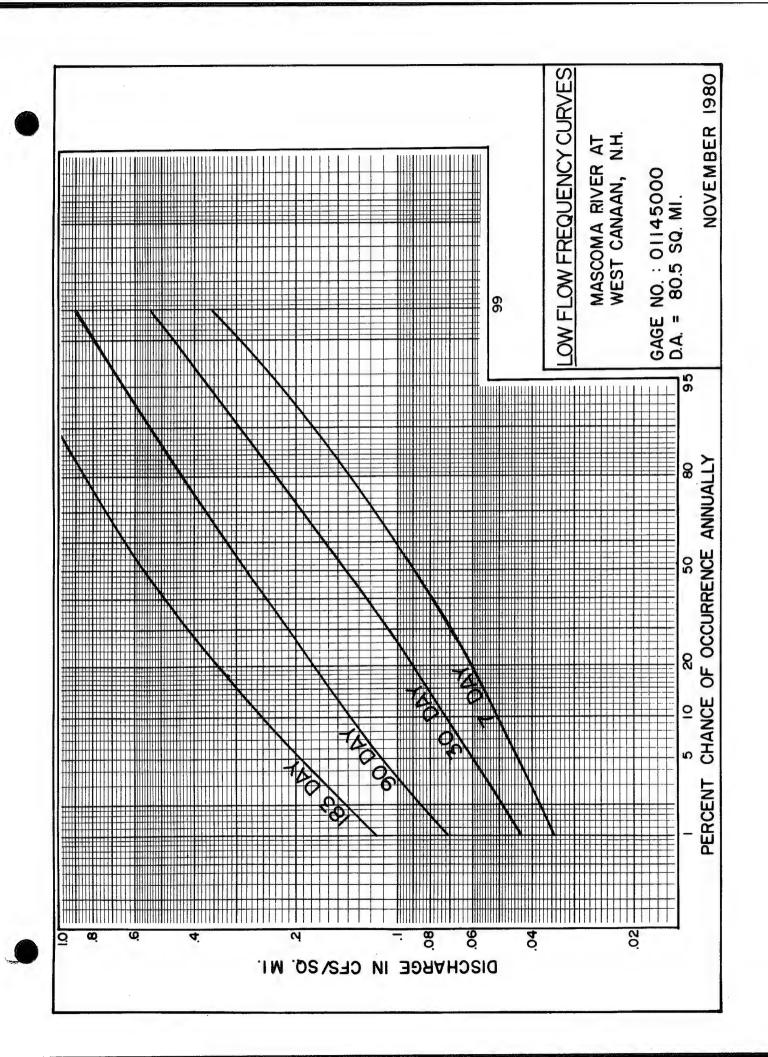


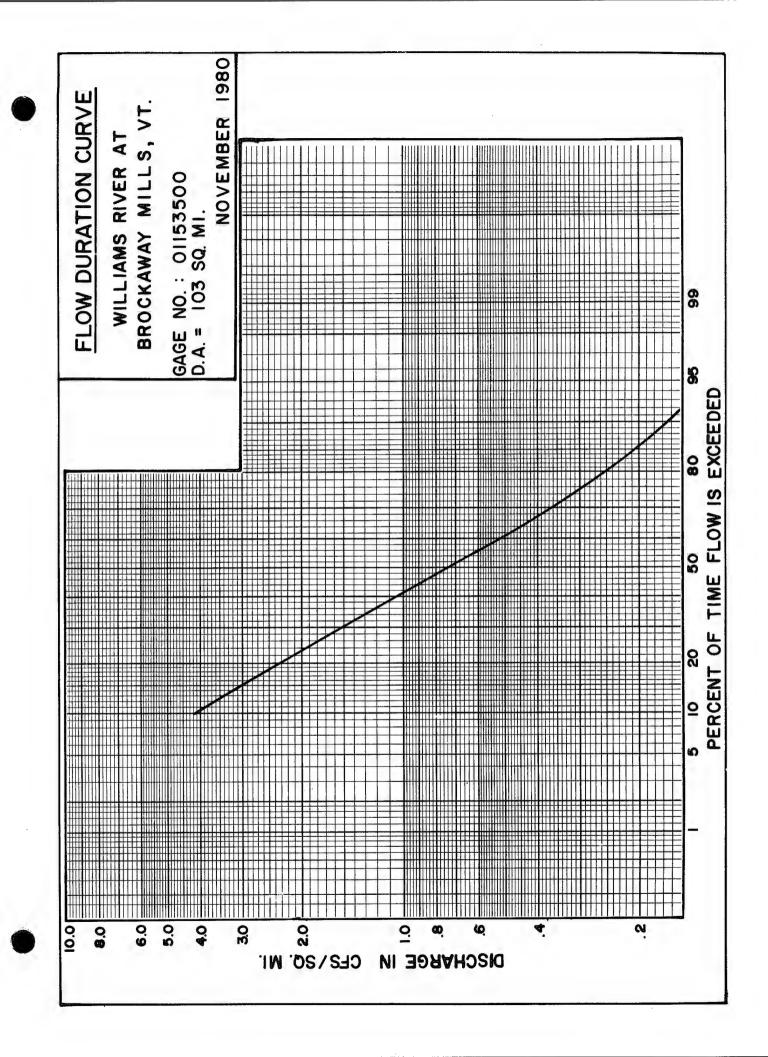


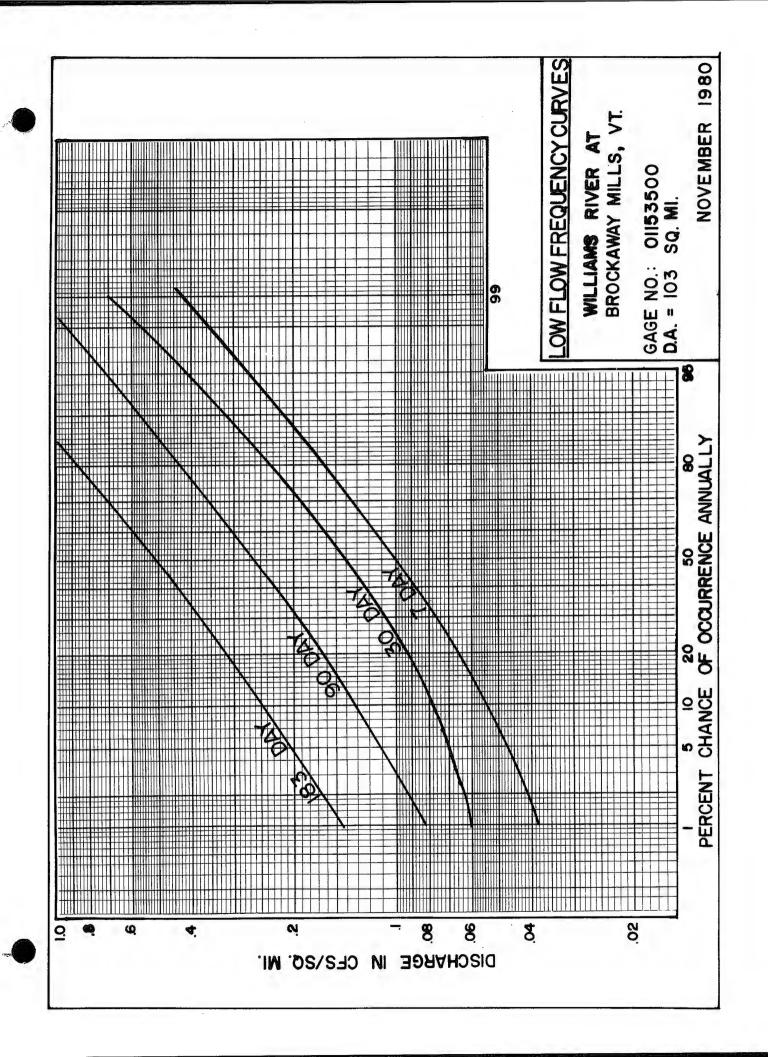


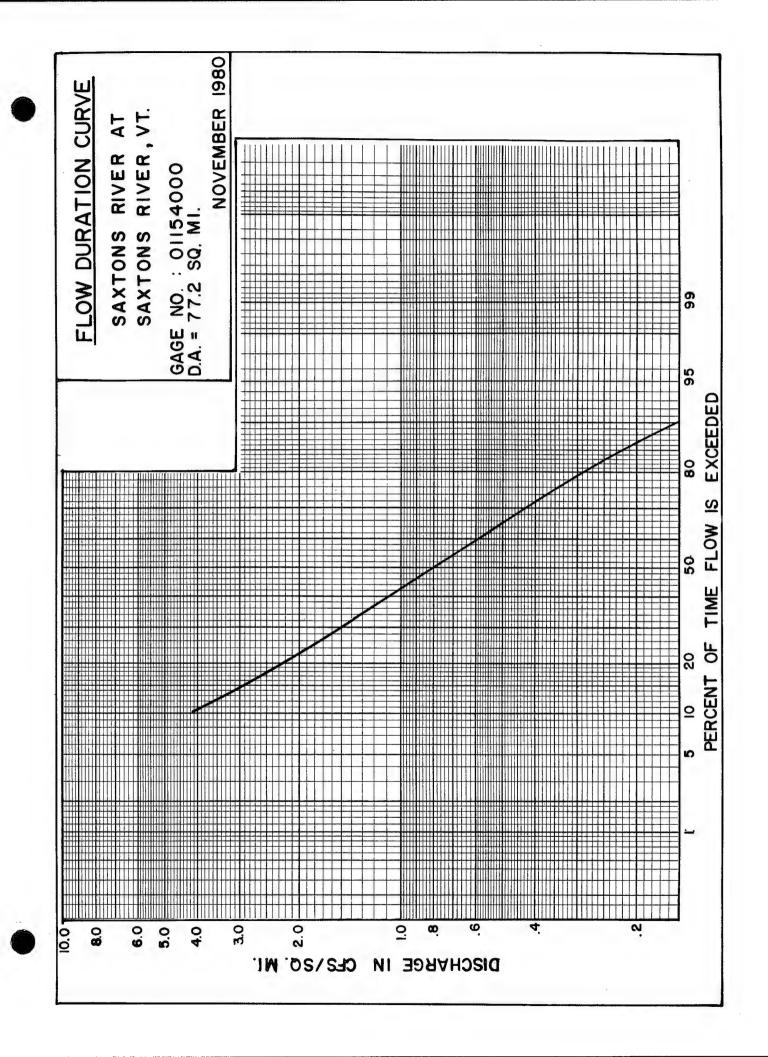


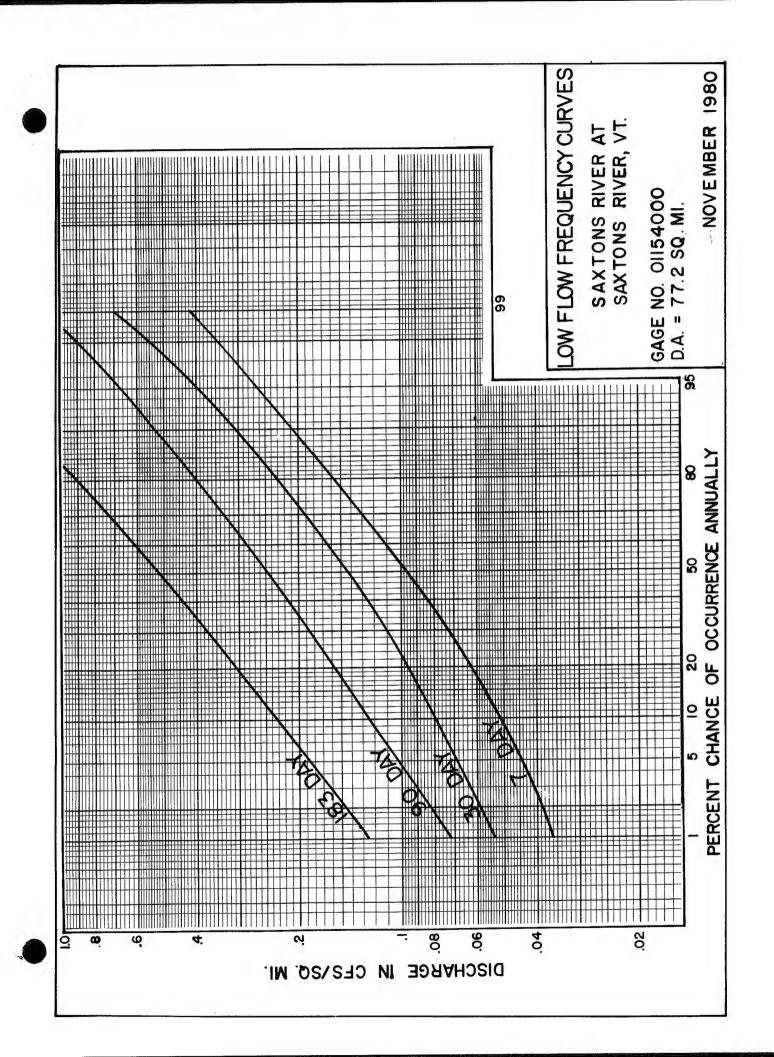


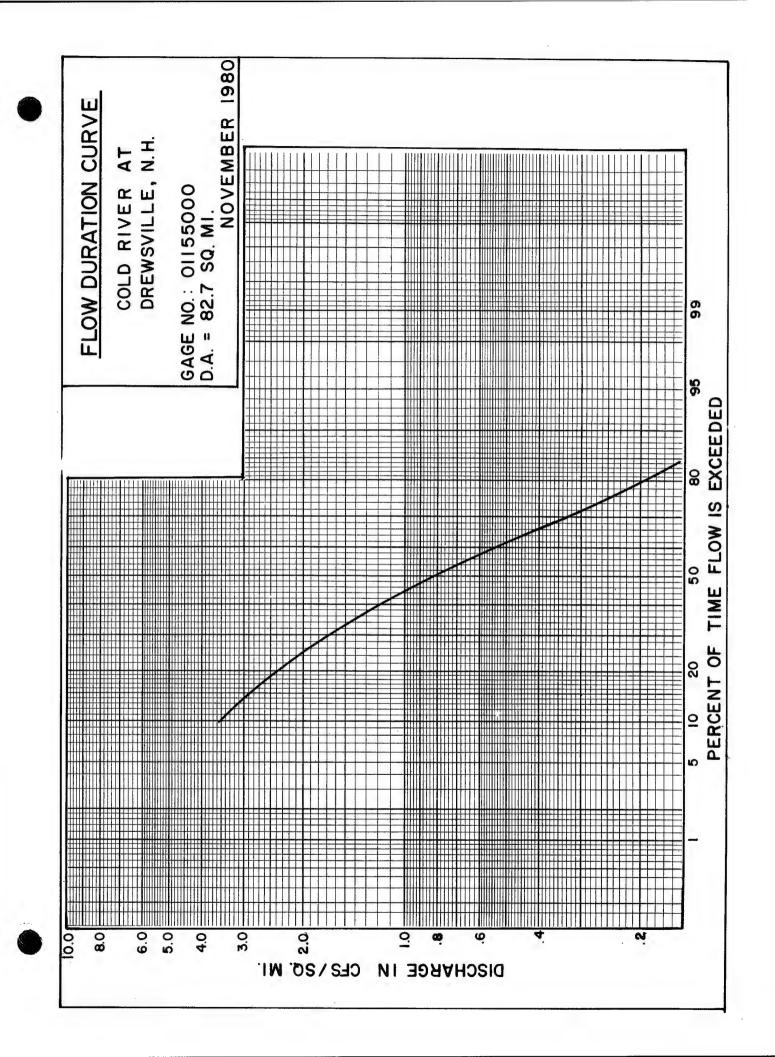


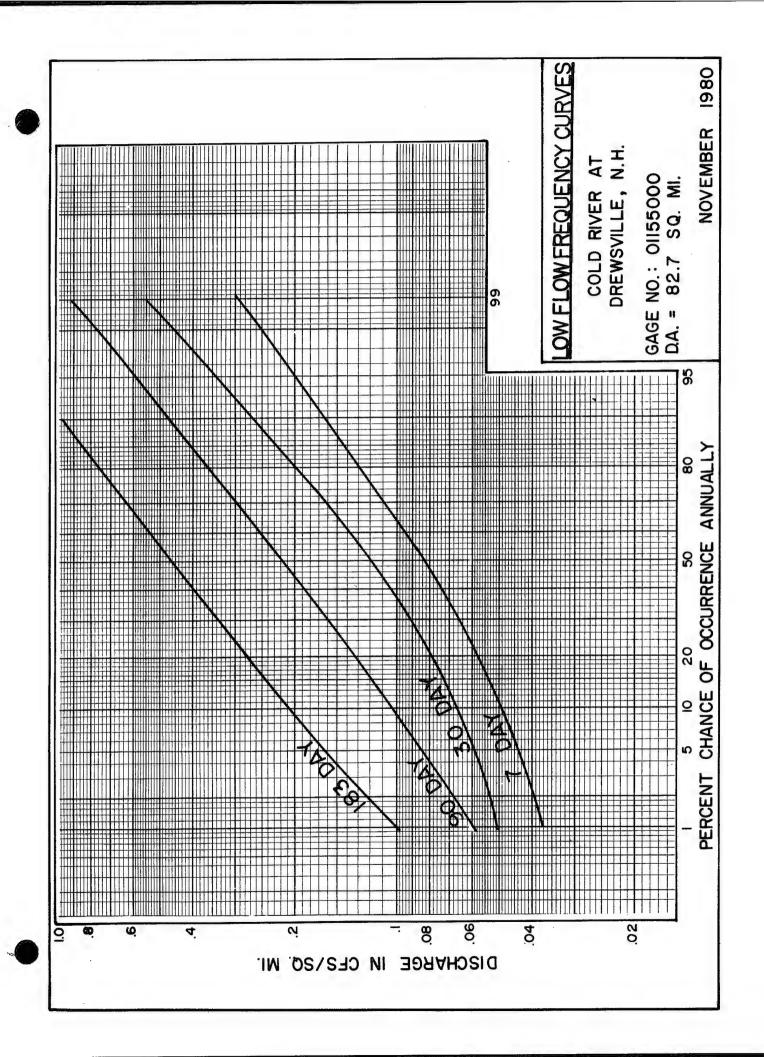


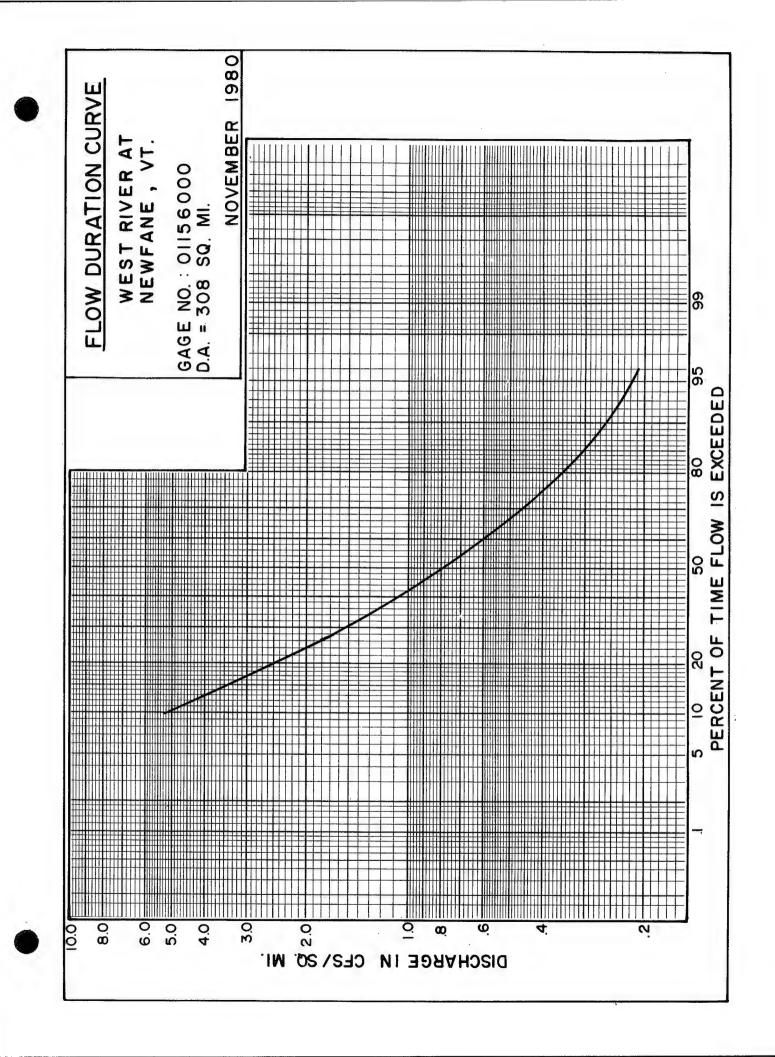


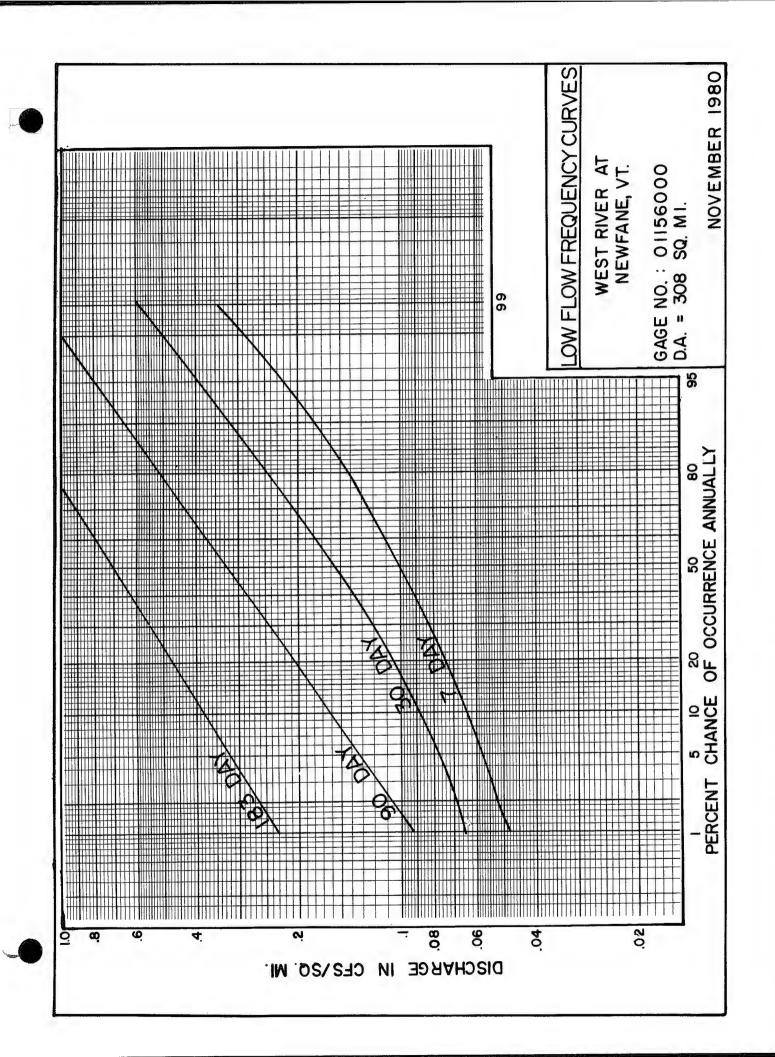


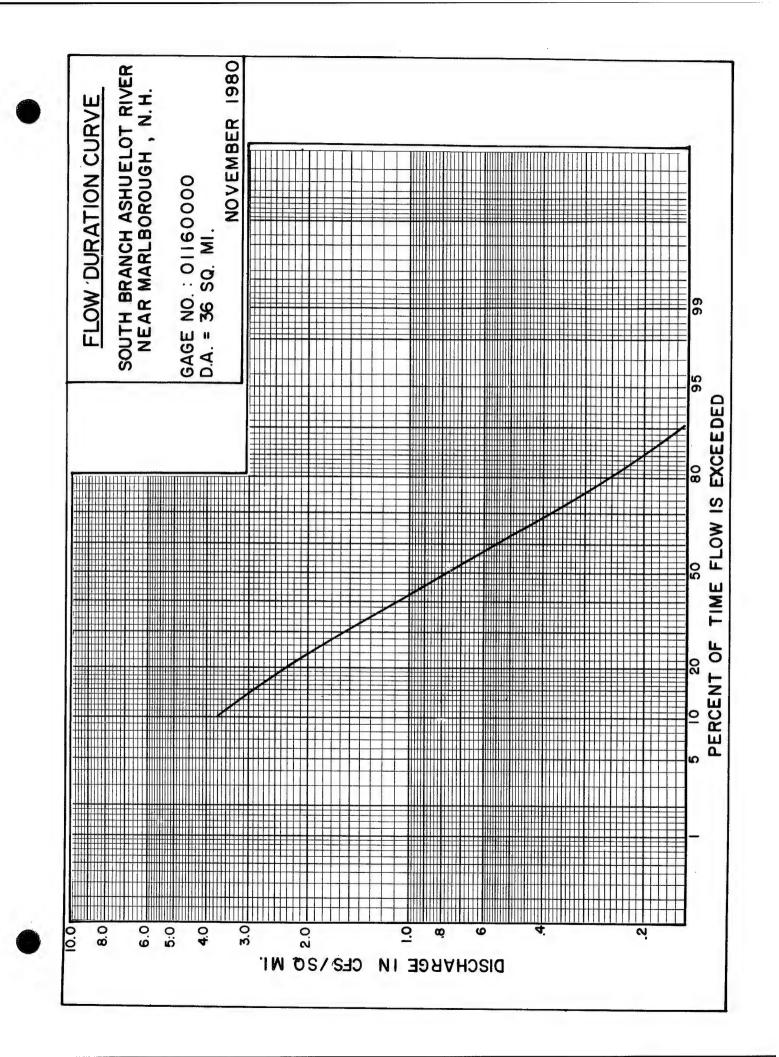


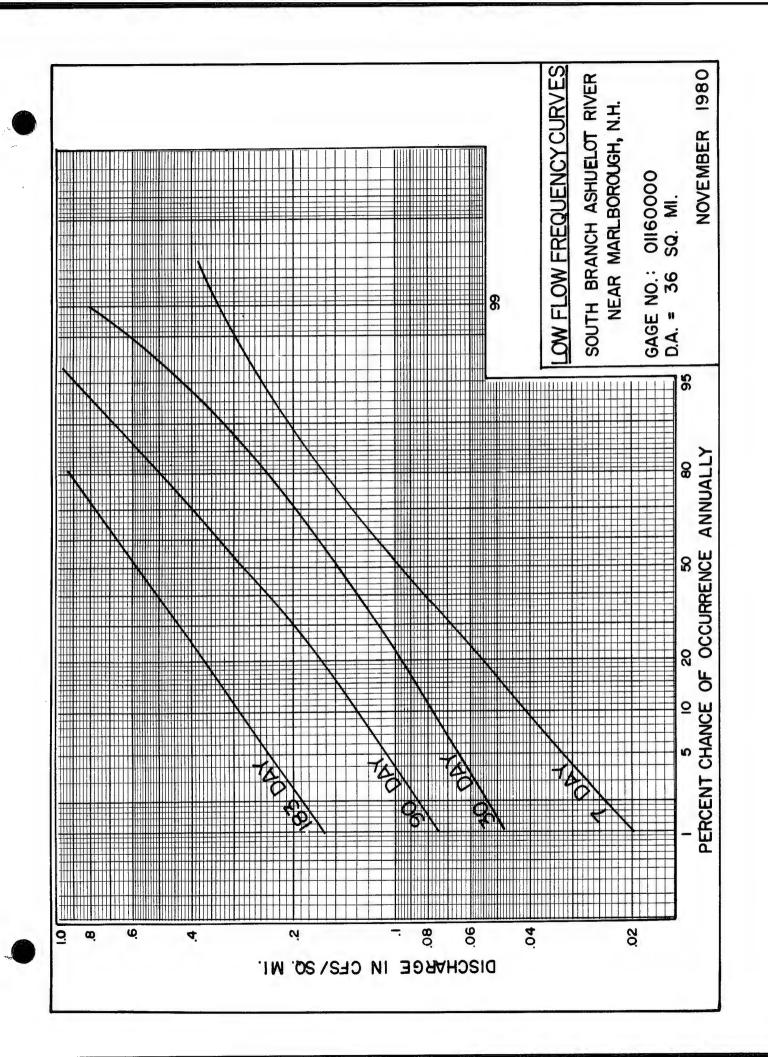












APPENDIX B
DISCHARGE DATA
USGS WATSTORE FILE

CONTENTS

Station No.	Name
01052500	Diamond River near Wentworth, NH
01054300	Ellis River at South Andover, ME
01057000	Little Androscoggin River near South Paris, ME
01064300	Ellis River near Jackson, NH
01064400	Lucy Brook near North Conway, NH
01064500	Saco River near Conway, NH
01064800	Cold Brook at South Tamworth, NH
01065000	Ossipee River at Effingham Falls, NH
01072850	Mohawk Brook near Center Stratford, NH
01073000	Oyster River near Durham, NH
01073600	Dudley Brook near Exeter, NH
01074500	E. Branch Pemigewasset River near Lincoln, NH
01075000	Pemigewasset River at Woodstock, NH
01075500	Baker River at Wentworth, NH
01075800	Stevens Brook near Wentworth, NH
01076000	Baker River near Rumney, NH
01076500	Pemigewasset River at Plymouth, NH
01077000	Squam River at Ashland, NH
01078000	Smith River near Bristol, NH
01080500	Lake Winnipesaukee Outlet at Lakeport, NH
01081000	Winnipesaukee River at Tilton, NH
01081500	Merrimack River at Franklin Jct., NH
01082000	Contoocook River at Peterborough, NH

Station No.	<u>Name</u>
01083000	Nubanusit Brook near Peterborough, NH
01084000	N. Branch Contoocook River near Antrim, NH
01084500	Beards Brook near Hillsboro, NH
01085000	Contoocook River near Henniker, NH
01085500	Contoocook River below Hopkinton Dam at W. Hopkinton, NH
01085800	W. Branch Warner River near Bradford, NH
01086000	Warner River at Davisville, NH
01087000	Blackwater River near Webster, NH
01089000	Soucook River near Concord, NH
01090800	Piscataquog River below Everett Dam, near E. Weare, NH
01091000	S. Branch Pisactaquog River near Goffstown, NH
01091500	Piscataquog River near Goffstown, NH
01092000	Merrimack River near Goffs Falls below Manchester, NH
01093000	Sucker Brook at Auburn, NH
01094000	Souhegan River at Merrimack, NH
01128500	Connecticut River at First Connecticut Lake near Pittsburg, NH
01129200	Connecticut River below Indian Stream near Pittsburg, NH
01129500	Connecticut River at North Stratford, NH
01130000	Upper Ammonoosuc River near Groveton, NH
01135000	Connecticut River near Dalton, NH
01133000	E. Branch Passumpsic River near East Haven, VT
01134500	Moose River at Victory, VT

Station No.	<u>Name</u>
01135000	Moose River at St. Johnsbury, VT
01137500	Ammonoosuc River at Bethlehem Jct., NH
01138000	Ammonoosuc River near Bath, NH
01141800	Mink Brook near Etna, NH
01142500	Ayers Brook at Randolph, VT
01144500	Connecticut River at West Lebanon, NH
01145000	Mascoma River at West Canaan, NH
01150500	Mascoma River at Mascoma, NH
01152500	Sugar River at West Claremont, NH
01153500	Williams River at Brockways Mills, VT
01154000	Saxtons River at Saxtons River, VT
01154500	Connecticut River at North Walpole, NH
01155000	Cold River at Drewsville, NH
01156000	West River at Newfane, VT
01157000	Ashuelot River near Gilsum, NH
01158000	Ashuelot River below Surrey Mt. Dam, near Keene, NH
01158500	Otter Brook near Keene, NH
01158600	Otter Brook below Otter Brook Dam, near Keene, NH
01160000	S. Branch Ashuelot River at Webb, near Marlborough, NH
01161000	Ashuelot River at Hinsdale, NH

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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN

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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 DISCHARGE, IN CUBIC FEET PER SECOND
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ELLIS RIVER NEAR JACKSON, NH

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ACCUM.	2488	2090	1783	1521	1271	1036	824	299	915	393	307	238
TOTAL	398	307	262	250	235	215	162	971	123	98	69	72
VALUE	19.0	23.0	27.0	32.0	0.63	46.0	55.0	65.0	78.0	93.0	110,0	130.0
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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 Hean

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LUCY BROOK NEAR NORTH CONWAY, NH	m	0.59	06.0	0.72	0.41	1.30	1.19	0.89	1.10	1.10	1.10	0.87	1.50	0.10	0.52
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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30

DISCHARGE, IN CUBIC FEET PER SECOND

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SOUND TO THE STATE OF THE STATE * TOOCCOC~~~~NN MOWANOBONPOMO

VALUE EXCEEDED "P" PERCENT OF TIME

DISCHARGE, IN CUBIC FEET PER SECOND
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SACO RIVER NEAR CONMAY, NH

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YEAR ENDING MARCH 31 Z CONSECUTIVE DAYS 9 FOLLOWING NUMBER THE FOR RANKING LOWEST MEAN VALUE AND DISCHARGE, IN CUBIC FEET PER SECOND 30

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472,00 5845 5845 600 413 517 600 760 760 413,00 531,00 560,00 877,00 646.00 705.00 801.00 N 0000 O TO 283.00 283.00 307.00 212.00 475.00 269,00 260,00 375,00 526,00 374.00 175,00 187 8 2 2 2 E 20 ちょうない M T M 244.00 244.00 377.00 377.00 215.00 240.00 192.00 162.00 352.00 47.3233 1333 40 200 188.00 201.00 175.00 187.00 207.00 233.00 276.00 318.00 287.00 155,00 9 29.2746 37 Agree 5 4 2364. 1 00 150.00 183.00 154.00 164.00 214 00 279 00 279 00 265 00 274.00 120.00 186.00 14 400 km 5 W 4 W W 25.5 163.00 163.00 175.00 175.00 135.00 137.00 148.00 273.00 103.00 18 823 8 41 151.00 131.00 152.00 172.00 171.00 101,00 125.00 147.00 142.00 165.00 00 **よってはなるとの** 0 m 0 m 0 m 252 98.00 116.00 139.00 120.00 137.00 124.00 147.00 170.00 212.00 135,00 254,00 128,00 RIVER NEAR CONWAY, NH ちゅうりょう 0 200 250 2 96.00 1138,00 138,00 117,00 136,00 122.00 143.00 165.00 203.00 130.00 SACO 1964 596 969 970 972 976 978 967

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VALUE EXCEEDED "P" PERCENT OF TIME" CFS

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150.00	180.00	260.00	290.00	450.00	00.086	2300,00
# 56A	3 001	V75 E	V70 =	V50 #	V25 #	× 017

STATION NUMBER 01064800

DISCHARGE, IN CUBIC FEET PER SECOND

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STATION NUMBER 01064800

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DISCHARGE, IN CUBIC FEET PER SECOND
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STATION NUMBER 01072850

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STATION NUMBER 01072850

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DISCHARGE, IN CUBIC FEET PER SECOND.

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31

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STATION NUMBER 01073000

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN MEAN OYSTER RIVER NEAR DURMAN, NH

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DISCHARGE, IN CUBIC FEET PER SECOND
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STATION NUMBER 01073600

LOWEST WEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN TOUGHE BROOK NEAR EXCLASS IN YEAR ENDING MARCH 31 DUDLEY BROOK NEAR EXETER, NH

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B-24

DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 DISCHARGE, IN CUBIC FEET PER SECOND MEAN DUOLEY BROOK NEAR EXETER, NH

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8-25

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN

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DURATION TABLE OF DÁILY VALUES FOR YEAR ENDING SEPTEMBER 30 MEAN FAST BRANCH PEMIGEMASSET RIVER NEAP LINCOLN, NH

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STATION NUMINE 01075000

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DISCHARGE, IN CUBIC FEET PER SECOND
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PEMIGEWASSET RIVER AT WOODSTOCK, NH

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STATION NUMBER 01075500

LOWEST HEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN

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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING S

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STATION NUMBER 01075800

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN

YEAR 1 10.05 1 0.010 1 0.02 4 0.02 1 0.04 2 0.07 1 0.11 1 0.15 1 0.25 1966 0.01 1 0.01 2 0.02 4 0.02 1 0.00 1 0.07 1 0.11 1 0.11 1 0.25 1966 0.01 2 0.01 2 0.02 9 0.08 8 0.11 5 0.17 6 0.26 6 0.26 6 0.26 6 0.27 1 0.17 6 0.26 6 0.27 1 0.18 1 0.18 1		•						M.						± 5			
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STATION NUMBER 01075800

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CISCHARGE, IN CUBIC FEET PER SECOND

MANCH 31

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3-37

STATION NUMBER 01076500

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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN HEAN PENDING MARCH 31 MEAN PENDER AT PLYMOUTH, MH

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DISCHARGE, IN CUBIC FEET FER SECOND
MEAN
PEMIGEWASSET RIVER AT PLYMUTH, NH

PERC	3	2			•							
ACCUM	1239	800	202	562	147	78	41	61	13	7	2	
TOTAL	439	295	206	152	63.	43	22	•	•	S	~	
VALUE	2100	6300	7700	9500	12000	14000	18000	22000	27000	33000	40000	
CLASS	77	25	56	27	28	56	30	31	32	33	34	
PERCT	67.6	58.4	50.5	43.0	36.3	30.8	25.1	21.0	16.9	12.4	9.3	9.9
ACCUM	18532	16011	13755	11781	6666	8778	6868	5764	0191	3400	2545	1797
TOTAL	2521	2256	1974	1842	1491	1580	1104	1124	1240	855	748	558
VALUE	430.0	530.0	650.0	.800.0	0.066	1200.0	1500.0	1800.0	2200.0	2800.0	3400.0	4500.0
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CONTE	180,00	230.00	370.00	410.00	650,00	1500.00	3300.00
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B-3

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 Mean

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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 DISCHARGE, IN CURIC FEET PER SECOND MEAN SQUAM RIVE! AT ASHLAND, NH

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TOTAL	1038	215	327	89	100	7.0	18	100	2	• •	-	•
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PERCT	7.66	49.4	66	99.2	98.4	97.9	97.3	7.96	95.7	73.1	400	
ACCUM	14529	14518	14514	14498	14373	14308	61671	14129	13986	10685	5896	
TOTAL	11	7	91	125	65	68	0.6	143	3301	4789	2548	
VALUE	0.0	12.0	15.0	18.0	23.0	28.0	35.0	0.87	52.0	65.0	79.0	
CLASS	12	13	14	15	16	17	1.8	0	20	25	22	
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B-43

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN SMITH RIVER NEAR BRISTOL, NH

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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30

DISCHARGE, IN CUBIC FEET PER SECOND HEAN SHITH RIVER NEAR BRISTOL, NH

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VALUE EXCEEDED "P" PERCENT OF TIME

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STATION NUMBER 01081500

DISCHARGE, IN CUBIC FEET PER SECOND
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STATION NUMBER 01082000

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31

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	14 27,00 29 6,90 3 13,00 18 6,90 4	12.00 13. 11.00 14. 3 6.50 14. 3 5.00 30	10.00 13 7.10 13 13.00 22 16.00 15	44. 44. 44. 44. 44. 44. 44. 44. 44. 44.	8 80 8 10 10 10 10 11 11 11 11 11 11 11 11 11	14.00 24 14.00 24 14.00 25 9.10 20
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STATION NUMBER 01082000

DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 HEAN CONTOCOOK RIVER AT PETERBORDUGH, NH

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ACCUM	1906	1367	000	547	302	174	. 69	200) 	•		
TOTAL	539	197	100	200	128	105	977	2.0	-	•		
VALUE	2000	250.0	320.0	410.0	520.0	650.0	830.0	1100.0	1300.0	1700.0	2200.0	
CLASS	7	52	92	27	28	6	30	1	2	1 10	7	
PERCT	95.3	92.0	86.8	80.8	75.4	69.3	62.0	54.5	0.04	37.8	30.4	22.2
ACCUM	11111	10748	10141	7776	8897	8097	7249	6371	54.86	4422	3554	2594
TOTAL	193	209	269	637	710	878	878	888	1064	40.00	096	688
VALUE	11.0	14.0	18.0	23.0	59.0	37.0	47.0	0.09	76.0	0.96	120.0	160.0
CLASS	12	13	7 7	15	16	17	1.8	61	02	2	22	23
PERCT	100.0	100.0	100.0	100.0	6.66	8.00	1.66	9.66	7.00	1.06	98,5	97.2
ACCUM	11688	11688	11686	11684	11676	11662	11657	11644	11621	11584	11508	11364
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DISCHARGE, IN CUBIC FEET PEH SECOND
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NUHANUSIT BROOK NEAR PETERBOROUGH, NH

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OROL	•	ž	5105	5105	5704	00	15692	13	09	31	15537	154	5388	550
ERB	10	ACCUM	157	157	157	157	156	. 56	156	156	155	154	153	15
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00K	••	<u> </u>	0	0	0	90	0	0	0,	06	01	0	30	80
11 83	•	VALUE	0.0	0	0.6	0.8	1.0	1.2	1.60	5	2	3.	3.80	7.77
MEAN NUBANUSIT BROOK NEAR PETERBOROUGH,	EAR 977	85.4			٠ ٨	3 90	· 3	5	•		- 40	•	0	
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VALUE EXCEEDED "P" PERCENT OF TIME.

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	u	*	14		H	u	60
	0	0	-	V70	5	2	-

01084000 STATION NUMBER

F MARCH YEAR ENDING Z DAYS CONSECUTIVE 90 NUMBER FOLLOWING THE FOR RANKING LOWEST MEAN VALUE AND CUBIC FEET PER SECOND Z

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10701

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B-60

STATION NUMBER 01084000

LOWEST HEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 Persy

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	30 3	0.37 4		1.00 8	1.50 7		11.00 18	18.00 23	
	10 19	2.20 20		3,10 19	6.10 24		21.00 30	28.00 32	
	70 15	1.80 15		2,20 12	3,20 12		9.10 12	18.00 24	
	30 21	2.40 21		5.10 27	14.00 40		39.00 41	43.00 39	

DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30

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RGE, IN CUBIC FEET PER S Branch Contoocook River	•	:	•		•			74115		. с	, =	, 0	. •			-		_	•
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DISCHARGE, IN CUBIC FEET MEAN NORTH BRANCH CONTODCOOK R	CLASS	1965	1966	1967	1969	101		A . A . C	,	•	• 11		. 7	· u		, .			
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VALUE EXCLEDED "P" PERCENT OF TIME

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0	0	-	-	0 S A	n	-

STATION NUMBER 01084500

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 DISCHARGE, IN CUBIC FEET PER SECOND

DISCHARGE, IN CUBIC FEET PER SECOND MEAN BEARDS BROOK NEAR HILLSBORD, NH

-	7 6 7								•	•				
7:00	*CCU3	3 6	1500	47.0	199	917	243	141	16	52	~	-4		
	TOTAL	1 th	613	315	545	173	102	65	51	1.8	•	-		
	VALUE	30.0	70.0	0.06	0.06	\$80.0	0.000	550.0	0.008	0.001	0.004	0.000		
	LASS		:								_	_	,	
•	3		*							-				
	PERCT	86.6	82,5	78.4	75.6	71.5	66.2	60.8	0.05	47.0	39.6	32.0	25,7	
	ACCUM	1001	7533	7162	#U69	652B	6709	5508	0000	4289	3613	2918	2351	
	TOTAL	374	371	25.8	376	010		010		174	0.0	767	000	
	ALUE	້າ	7.2	7.6	12.0	0	- 0	0 10		100	000	78.0	100.0	
	LASS	12	13	7 7					0		200		N C	
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:	PFFCT	100.00	1000	100	0.00		44.00		7 6	0.40	0 0	0 10	700	
	ACCUM	9131	9131	0110	40.0		0000	7707	100		C + C C	0 A		
	TOTAL	•	, -	• •			E 6	9	0 1		100	200	444	
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	VALUE EXCEEDED)	× 8 567	# C6A	V75 8	V70 B			

DISCHARGE, IN CUBIC FEET PER SECOND
FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31
FORTOCORD BILLS IN CUBIC FEET PER SECOND

CONTOOC	OOK RIVER NEA	IR HENNIKER, NH								
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70	0.6	00.9	0.	75,00 18	•	2 00 6		2 00.	000	D 1
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76	000	9.00 3	9,00 3	1.00 3	87.00 3	2.00 3	0.00	15.00 2	200000	7
9	7.00 2	4.00 2	82.00 2	87.00 Z	00.00	32,00 2	57.00 2	66.00 2	0000	S.
1945	6	104,00 35	0 3	0000	* 00 3	82.00	18,00 3	11.0	72,00 3	0
ć			2 00	5000	78.00	19.00 3	24.00 3	69.00 3	09.00	
7 :	200	000	100		15 00 4	2 00 96	00 - 00	06.00 2	1 00 71	
7	2 000	7 000	2000	2000			30.00	51.00.1	8-1-00 1	
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9	0.0	-	8.00 2	8,00 2	2 00 5	2 00 5	1 00 001	0000		7
1					2000	800	1 00 1	29.00.1	08.00	
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95	5.00 2	8.00 2	3.00.2	80.00 2	9	165.00 63	u	137.00 13		
95	7.00 1	00.	41.00	00.67	10.00	81.00	00.26	99.00	00.	-
5561.	112,00 37	183,00 37	210.00 37	252,00 37	.00	77,00 3	63.0	\$ 00.08	11.00	
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U	000	1 000 1	0	000				8.6 00	50.00	46
G.	1 06.1	00.0	00.0	43.00	55.00		1000	4	200.00	, -
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97	Z 00 2	2000 2		000	000	•				

STATION NIJMRER 01085000

DISCHARGE, IN CUBIC FEET PER SECOND
HEAV
CONTODCOOK RIVER NEAR HENNIKER, NH

CLASS VALUE TOTAL ACCUM PERCT CLASS VALUE TOTAL ACCUM													
19.0 0 13880 100.0 13 170.0 700 11181 80.6 24 1300.0 11181 19.0 8 13880 100.0 13 170.0 96.0 461 10481 75.5 25 1600.0 113872 99.9 14 210.0 96.0 68.4 26 1900.0 2 27 2300.0 2 24.7 2300.0 2 22 2 2301.0 2496 224.7 230 20.0 2 22 2 2301.0 2496 224.7 230.0 24.7 250.0 24.7 250.0 24.7 250.0 24.7 250.0 24.7 250.0 25		STAIL	VALUE	TOTAL	ACCUM	PERCT	CLASS	VALUE	TOTAL	PERCT	CLASS	VALUE	TOTAL
1 19.0 8 13880 100.0 13 170.0 961 10481 75.5 25 1600.0 2 23.0 4 13872 99.9 14 210.0 960 960.4 260.0 660.4 27 2300.0 2 27.0 18 13860 99.9 15 250.0 867 469.9 7671 55.3 27 2300.0 2 2 2700.0 673 460.4 260.4 20.0 185 13797 99.4 17 360.0 873 6714 460.4 20.0 193 13612 96.1 18 430.0 875 6714 460.4 20.0 193 13612 96.1 18 430.0 667 4096 29.5 31 4800.0 67 10 100.0 567 4096 29.5 31 4800.0 67 10 100.0 567 30.0 667 30.	•				1480	100	21	100.0	100	80.6	74	1300.0	240
2 23.0				• •			1 M	1700	- 0	 75.5	500	1600.0	321
2 27.0 18 13672 99.9 15 250.0 66.5 27 2300.0 27 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 250.0 61.5 25 25 25 25 25 25 25 25 25 25 25 25 25		-	0 % 1	0	00001	000	7 :	0			4	000	077
3 27.0 18 13868 99.9 15 250.0 869 8540 61.5 27 2300.0 453.0 53 13850 99.8 16 300.0 957 7671 55.3 28 2700.0 53 13612 98.1 18 430.0 873 6714 48.4 22 3300.0 7 57.0 310 13419 96.1 18 430.0 875 65841 42.1 30 4000.0 7 57.0 340 13109 94.4 20.0 647 4096 29.5 31 4800.0 99.0 10 100.0 585 12255 88.3 100.0 496 2341 16.9 34 8300.0	4	~	63.0	3	13872	0	7	210.0	040		0 1	0000	- 1
4 33.0 53 13850 99.8 16 300.0 957 7671 55.3 28 2700.0 5 40.0 185 13797 99.4 17 360.0 873 6714 408.4 29 3300.0 5 40.0 13419 96.7 19 520.0 875 4966 31 4800.0 7 57.0 310 13419 96.7 19 520.0 870 4966 29.5 31 4800.0 8 40.0 520.0 870 4066 29.5 32 5700.0 9 83.0 496.3 22 910.0 512 2853 20.6 34 8300.0 10 100.0 496 2341 16.9 34 8300.0 11 120.0 496 2341 16.9	9	107	27.0	18	13868	0 60	1.5	250,0	869	61,5	27	2300.0	561
5 40.0 185 13797 99.4 17 360.0 673 6714 48.4 29 3300.0 7 57.0 193 13612 98.1 18 430.0 875 5841 42.1 30 4000.0 7 57.0 310 13419 96.7 19 520.0 870 4966 35.8 31 4800.0 8 63.0 496 29.5 35 5700.0 9 63.0 490 12745 91.8 21 760.0 576 3429 24.7 33 6900.0 10 100.0 585 20.6 34 8300.0 11 120.0 489 11670 84.1 23 1100.0 496 2341 16.9		1 2	2.4	2	14850	900	1.6	300.0	150	55,3	28	2700.0	504
5 48.0 193 13612 98.1 18 430.0 875 5841 42.1 30 4000.0 7 57.0 310 13419 96.7 19 520.0 870 4966 35.8 31 4800.0 8 65.0 55.0 55.0 55.0 55.0 55.0 55.0 55.0		v	100		11707	00	1.4	160.0	873	48.4	52	3300.0	112
7 57.0 310 13419 96.7 19 520.0 870 4966 35.8 31 4800.0 A 59.0 364 13109 94.4 20 650.0 667 4096 29.5 32 5700.0 9 83.0 490 12745 91.8 21 760.0 576 3429 24.7 33 6900.0 10 100.0 585 12255 88.3 22 910.0 512 2853 20.6 34 8300.0 11 120.0 489 11670 84.1 23 1100.0 496 2341 16.9	0	1	9 4	10	12412	0		0.074	27.5	42.1	30	0.0004	. 61
A 59.0 364 13109 94.4 20 630.0 667 4096 29.5 32 5700.0 683.0 490 12745 91.8 21 760.0 576 3429 24.7 33 6900.0 10 100.0 585 20.6 34 8300.0 11 120.0 489 11670 84.1 23 1100.0 496 2341 16.9		•		7	12010	0.40	0	0.000	4	35.8	31	4800.0	30
9 63.0 490 12745 91.8 21 760.0 576 3429 24.7 33 6900.0 10 100.0 5853 20.6 34 8300.0 11 120.0 489 11670 84.1 23 1100.0 496 2341 16.9		- d		2 4 5	11100	7 70		630.0	144	56.8	32	5700.0	15
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11 120.0 489 11670 84.1 23 1100.0 496 2341 16.9		0	100	585	12255	88.3	22	9100	512	50.6	T M	8300.0	••
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VALUE EXCEEDED "P" PERCENT OF TIME

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CONTOCCOUR R BL	R BL				HOPKINTON DAM AT M HOPKINTON.	EZ Z		i		-					i .		!		
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970	67.00	15	72.00	13	89.00	7.5	116.00		146.00		1.5		14	258.0		322,			365,0
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972	20.00	117	30.00	~	36.00	N	57,(96		16	00.00	0	124.0		136		,	167.0
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470	60.00	75	93.00	2	98.00	5	103,00	21 00	120.00		-	131.00	10	191.0	-	220.	00		418.0
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DISCHARGE, IN CUBIC FEET PER SECOND
HEAN
CONTOOCOOK R BL HOPKINTON DAM AT W HOPKINTON, NH

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50	7	MO	2	17.	=======================================	13	j	90	54	7	2 2	
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26	07	3 00	4	. ao	M	40 0	200	7	17	S	~ •	
25	=	. 	J 20	10	•	51	2	=	30	•	T 7	•
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18	20.5	127	4 2 2	M	22	40	50	27	·w	17	70	
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YEAR ENDING MARCH 31 DAYS IN FOLLOWING NUMBER OF CONSECUTIVE HE FOR DISCHARGE, IN CUBIC FEET PER SECOND

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B-76

DURATION TABLE OF DAILY VALUES + OR YEAR ENDING SEPTEMBER 30

DISCHARGE, IN CUBIC FEET PER SECOND
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SOUCOOK RIVER NEAR CONCORD, NH

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PERCT	72.7	68.9	63.6	58.6	52.9	46.3	0.01	33,3	28.0	22.4	18.1	12,9
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TOTAL	393	545	507	200	674	979	685	546	574	434	535	329
VALUE	23.0	28.0	35.0	43.0	53.0	65.0	80.0	0.66	120.0	150.0	180.0	230.0
CLASS	12	12	7	15	1.6	1.7	8	19	20	21	22	23
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PERCT	100.0	1000	100.0	9 66	78.7	4 16	95.6	93.3	89.3	86.3	81.6	79.7
ACCUM	10227	10227	10222	10181	10097	9966	9775	9543	9136	8830	8349	7844
TOTAL		יט	4.5	94	131	191	27.2	107	306	187	505	707
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VALUE EXCEEDED "P" PERCENT OF TIME

B-77

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 HEAN PISCATAQUOG RIVER BL EVERETT DAM, NR E WEARE, NH

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DISCHARGE, IN CUBIC FEET PER SECOND
HEAN
S BRANCH PISCATAGUOG RIVER NEAR GOFFSTOWN, NH.

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DISCHARGE, IN CUBIC FEET PER SECOND
HEAN
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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 HEAN

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DISCHARGE, IN CUBIC FEET PER SECOND
HEAN
CONNECTICUT R AT FIRST CONH LK NR PITTSBURG, NH

EXCEEDED 'P' PERCENT OF TIME EXCEEDED 'P' PERCENT OF TIME	CLASS	VALUE	TOTAL	ACCIJM	PERCT		LASS	VALUE	TOTAL		PERCT	CLASS	VALUE VALUE	TOTAL	ACCUM.	PER
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426 99.0 16 62.0 454 13923 61.5 28 690 557 901 605 97.4 13923 61.5 29 850 189 344 92.5 95.5 95.5 90.1 109 1589 95.1 1589 95.1 1589 96.1 1389 94.1	(44)	4.50	133	22559	9.66		15	50.0	799		7.79	27	570	266	1467	•
0.6.5 97.4 17 75.0 574 13469 59.5 29 850 189 344 55.9 95.1 10.00 10.9 15.9 15.9 95.1 10.00 10.8 15.8 95.9 30 10.00 10.9 15.9 95.1 10.0 17.8 11.5 2.4 3.1 15.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1	3	5.50	363	22420	0.66		16	62.0	454		61,5	28	069	557	901	~
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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 MEAN

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DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 MEAN CONNECTICUT BL INDIAN STREAM NR PITTSBURG, NH.

PERCT 27.3	19.0	15.6	7.1	0.7	30 °			N.	!		1
ACCUM			,						90	-	1
TOTAL	281	712	292	186	88	36	77	•	-	-	
VALUE	910	1000	1200	1400	1600	1800	2100	2400	2800	3300	:
CLASS 24	25	26	. 27	88	29	30	31	32	3.5	34	
					-	•	~		2		
PERCT	83.7	81.1	78.	75.7	72.	67.6	61.	56	5.25	45.	34.
ACCUM.	7032	6811	6557	6562	6083	5676	5201	4712	4582	3500	2876
TOTAL	721	254	195	279	407	475	489	330	882	624	585
VALUE	160.0	190.0	220.0	250.0	290.0	340.0	390.0	450.0	510.0	290.0	680.0
CLASS	 	17	15	16	17	18	61	20	21	22	23
	.										:
PERCT	100	8 66	5 66	98.7	97.4	0.96	2.76	93.3	7.10	89.8	88.5
ACCUM	8400	8384	8355	8287	8182	6067	7956	7833	7701	7540	.7432
TOTAL	9	50	99	105	115	111	123	132	191	108	661
VALUE	30.00	35.00	40.00	46.00	53.00	61.00	70.00	81.00	93.00	110.00	120,00
CLASS	·		, I-4	3	S	•	-	•	0	10	1.1

VALUE EXCEEDED "P" PERCENT OF TIME

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	20	20			14	13
564	0	-	-	5	V25	•

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 Mean

DISCHAR	RE, IN CUBIC	DISCHARGE, IN CUBIC FEET PER SECOND			:			.:	:
CONVECT	FICUT RIVER	MEAN CONNECTICUT RIVER AT NORTH STRATFORD, NH	HN.		teneralistic control of the control			:	
YEAR	-		1		30	09	0.6		185
1471	183.00 17		214.00 13		369,00 17	417,00 10	498.00 11	592,00 15	865,00
1972	165.00 14		226.00 19		304.00 6	426,00 12	524.00 17	-	752.00
1973	61 00 561 .		325,00 33		519,00 38			~	1069.00 52
1974	335.00 42		512.00 44	,	633.00 44		1090,00	1260.00 47	30.000
1975	307.00 40	307,00 40 332,00 39	343.00 37	378,00 34	414.00 27	551.00 25	680,00 30	785.00 29	1000.000
1076	1111.00 2	116.00 1	128.00 1	148.00 1	248.00 3	401,00	İ	648,00 21	1260.00 4
1977	555.00 48	36.00 48	687.00 48	720.00 48	891.00 48	1000,000 46	1080,00 45	1150.00 45	1420.00 4
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LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 HEAN UPPER AMMONDOSUC RIYER NEAR GROVETON, NH

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DAILY VALUES . OR YEAR ENDING SEPTEMBER 30

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STATION NUMBER

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LOWEST HEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS. IN YEAR ENDING MARCH 31

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DISCHARGE, IN CUBIC FEET PER SECOND DURATION TABLE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30
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MASCOMA RIVER AT MASCOMA, NH

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DURATION 146LE OF DAILY VALUES FOR YEAR ENDING SEPTEMBER 30 STATION NUMBER DIISADO DISCHARGE, IN CUSTC FEET PER SECTION SALTONS RIVER, VI

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1 HARCH ENDING YEAR Z DAYS CONSECUTIVE OF NUMBER FOLLOWING H F08 RANKING PER SECOND CUBIC FEET PFD Z I SCHARGE,

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DISCHARGE, IN CUBIC FEET PER SECOND
HEAN
CONNECTICUT RIVER AT NORTH WALPOLE, NM.

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STATION NUMBER . 01:155000

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LOWEST MEAN VALUE AND RANKING FIR THE FOLLOWING NUMBED HE CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 DISCHARGE, IN CURIC FEET PER SECOND MFAN

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included in low Flow Frequency analysis for this gage. It was assumed that the ettert required to incorporate this From years 1921 to 1923 were not of 1930 - 196: would not improve the resulta by three years : lo the gage analysis For the period J.MR Significant amount Gage .. clata

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31

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STATION NUMBER 01158500

LOWEST MEAN VALUE AND RANKING FOR THE FOLLOWING NUMBER OF CONSECUTIVE DAYS IN YEAR ENDING MARCH 31 DISCHARGE, IN CURIC FEET PER SECOND

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STATION NUMBER 01158500

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